

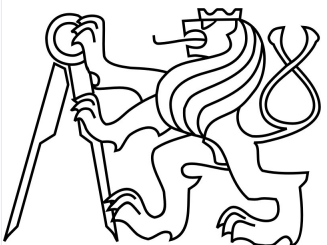
Analysis of Curved-Fibre Composites

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Contents:

1. Analysis
2. Stress and deformation
3. Tensor calculus
4. Deformation tensor and Energy minimum principles
5. The illustrating problems
6. Stiffness maximization

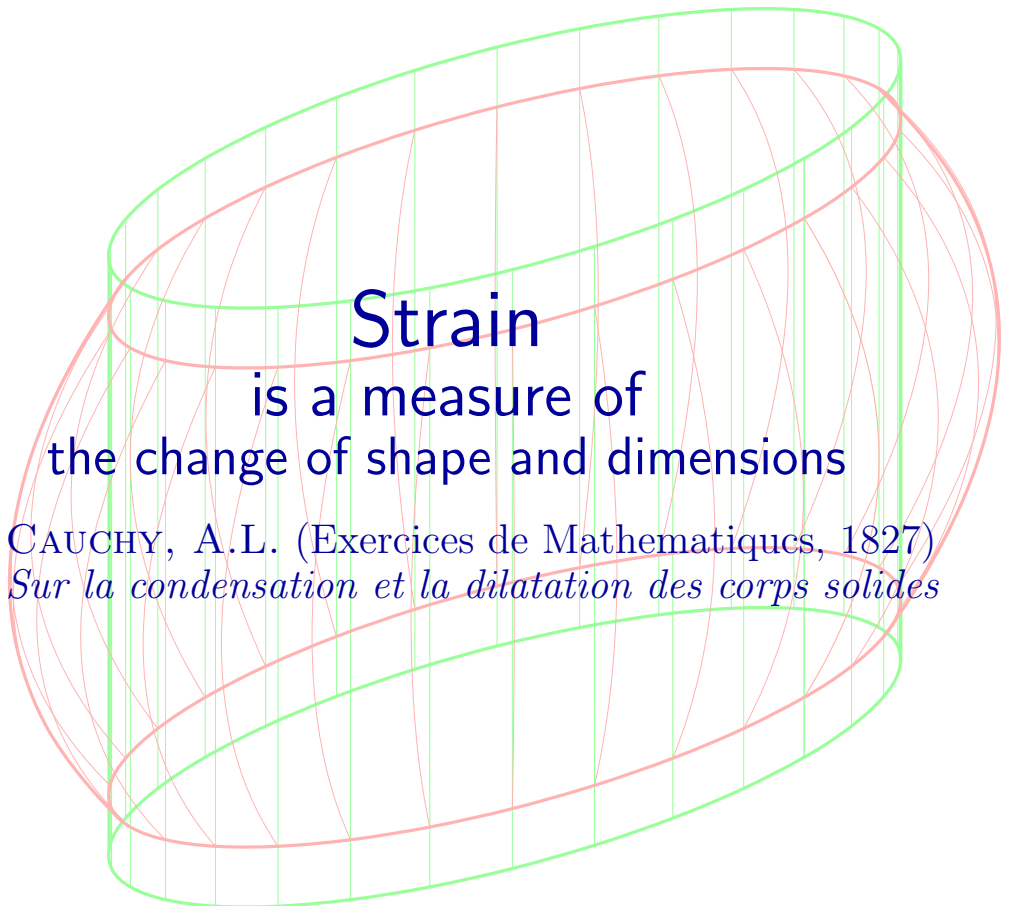


Analysis of deformation

Analysis = *Ανάλυσις*

in Greek

The action of
taking something apart
in order to study it



Strain
is a measure of
the change of shape and dimensions

CAUCHY, A.L. (Exercices de Mathematiques, 1827)
Sur la condensation et la dilatation des corps solides

We are used to
connect deformation with an action of forces

This leads us to the
concept of Elastic body



SIR WILLIAM PETTY (London, 1674)
... a new Hypothesis of Springing or Elastique Motions

ROBERT HOOKE (London, 1678/1660)
De Potentiâ Restitutiva

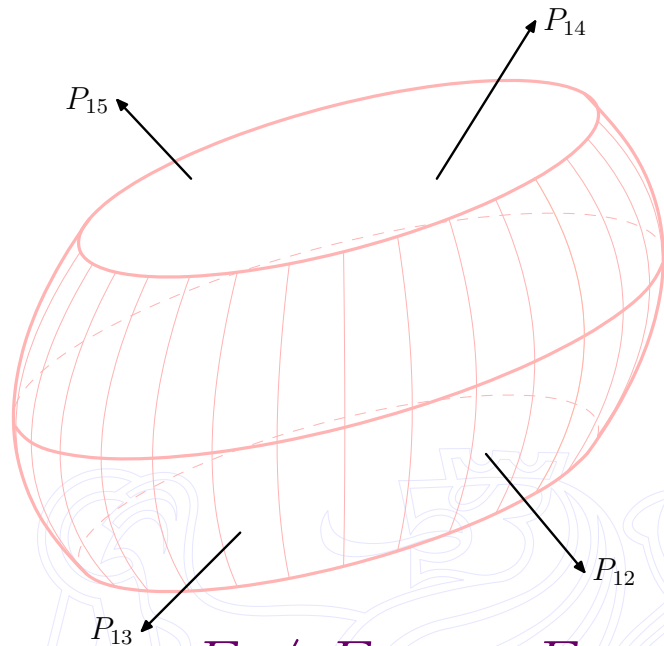
Stress

If we accept that

$$\text{deformation} = f(\text{acting forces})$$

we must look at the

description of these forces



$$F = \int_A \mathbf{t} dA$$

where the traction

CAUCHY, A. L. (1823)

$$\mathbf{t} = \lim_{A \rightarrow 0} \frac{\mathbf{F}}{A}$$

$$F \neq F_1 \Rightarrow F = f(A)$$

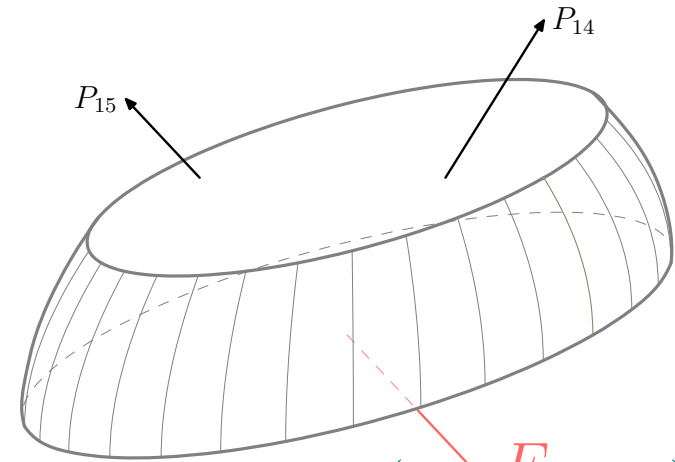
$$F \neq P \Rightarrow F = g(\mathbf{n})$$

\mathbf{n} — outer normal

For $A \rightarrow 0$ we define stress σ as

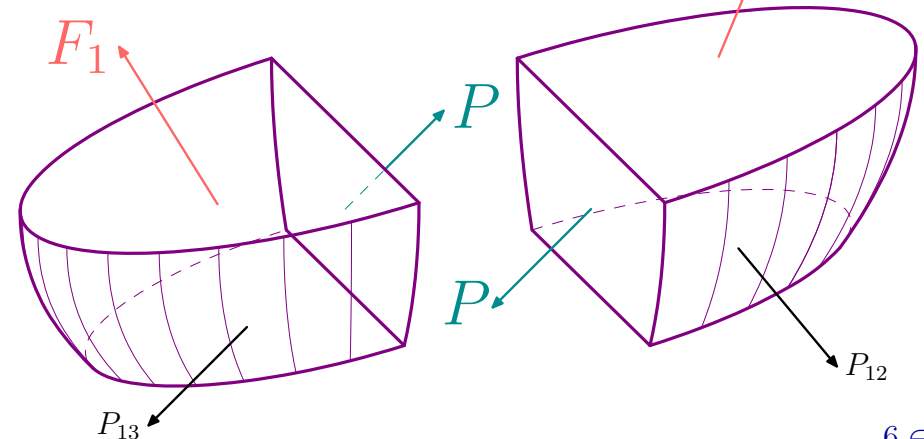
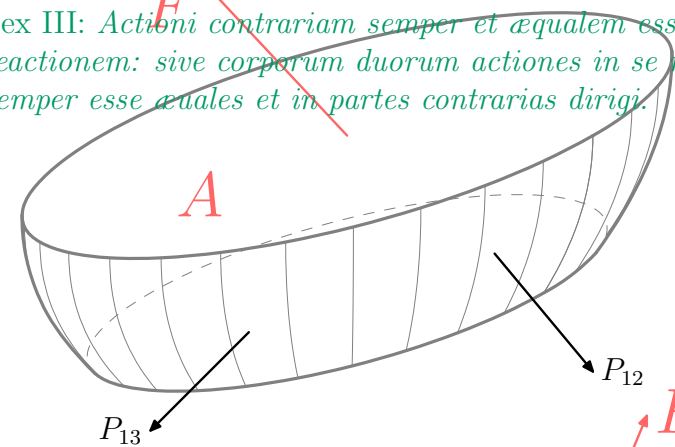
$$F = \mathbf{t}A = \sigma(\mathbf{A}), \quad \mathbf{A} = A\mathbf{n}$$

it turns out that—



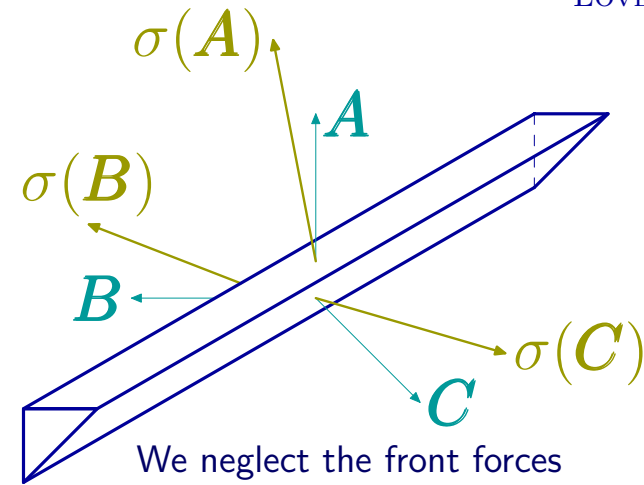
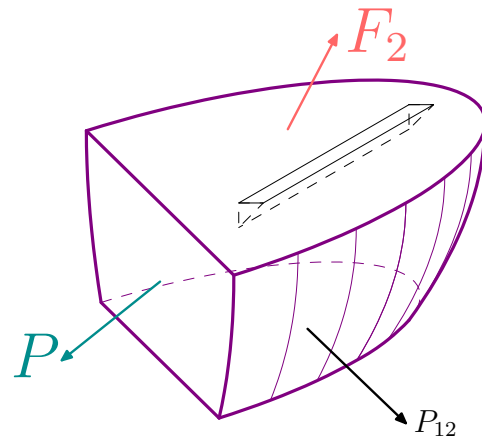
ISAAC NEWTON (1642—1727)

Lex III: *Actioi contrariam semper et æqualem esse reactionem: sive corporum duorum actiones in se mutuo semper esse æuales et in partes contrarias dirigi.*



Stress tensor

Take out
a long element



The total force

$$\mathbf{F} = \sigma(\mathbf{A}) + \sigma(\mathbf{B}) + \sigma(\mathbf{C})$$

$\mathbf{A}, \mathbf{B}, \mathbf{C}$ — normals with length of the cross-section area
 $\sigma(\mathbf{A})$ — the force on the $\mathbf{A} = A\mathbf{n}$, similarly $\sigma(\mathbf{B}), \sigma(\mathbf{C})$

$$\lim_{x \rightarrow 0} \frac{\text{area}(x^2)}{\text{volume}(x^3)} = \infty$$

$$\frac{\text{area}}{\text{volume}} \propto \frac{\mathbf{F}}{\text{mass}} = \text{acceleration}$$

$$(\mathbf{F} \neq \mathbf{0} \Rightarrow \text{acceleration} \rightarrow \infty) \Rightarrow \mathbf{F}' = \mathbf{0}$$

$$\sigma(\mathbf{A}) + \sigma(\mathbf{B}) + \sigma(\mathbf{C}) = 0$$

$$\sigma(-\mathbf{C}) = -\sigma(\mathbf{C}) = \sigma(\mathbf{A}) + \sigma(\mathbf{B})$$

From the definition: $\sigma(k\mathbf{A}) = k\sigma(\mathbf{A})$

Geometrie: $\mathbf{A} + \mathbf{B} + \mathbf{C} = 0$

$$\sigma(\mathbf{A} + \mathbf{B}) = \sigma(\mathbf{A}) + \sigma(\mathbf{B})$$

The stress σ is a (linear) vector operator, *i.e.* **tensor**

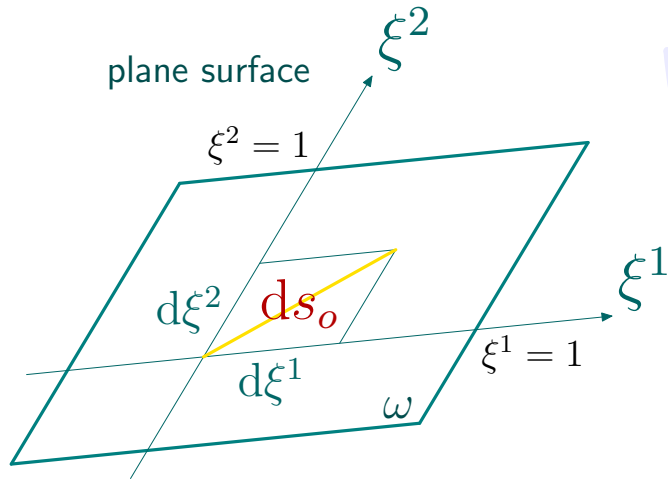
in abstract index notation:

$$F^a = t^a A = A \sigma^{ab} n_b \quad \text{tedy} \quad t^a = \sigma^{ab} n_b$$

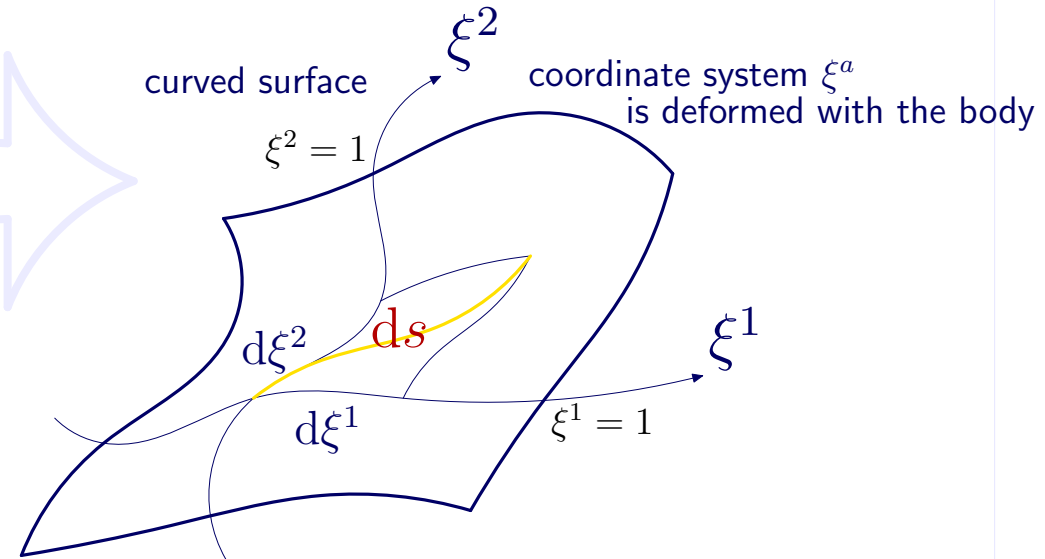
Tensor calculus

Distance of two points on a surface

- HAMILTON, W. R. (1854, 1855)
- WOLDEMAR VOIGT (1899)
- GREGORIO RICCI-CURBASTRO (1890)
- TULLIO LEVI-CIVITA (1900)
- ALBERT EINSTEIN (1915)
- SYNGE, J. L. and SCHILD, A. (1978)
- LOVELOCK, D. and RUND, H. (1989)



$$\theta : \omega \rightarrow \mathbb{R}^3$$



$$ds_o^2 = (d\xi^1)^2 + (d\xi^2)^2$$

$$ds_o^2 = \begin{pmatrix} d\xi^1 & d\xi^2 \end{pmatrix} \begin{pmatrix} 1 & 0 \\ 0 & 1 \end{pmatrix} \begin{pmatrix} d\xi^1 \\ d\xi^2 \end{pmatrix}$$

$$ds_o^2 = \delta_{ab} d\xi^a d\xi^b \quad (\text{Einstein summation})$$

δ_{ab} is Kronecker symbol

LEOPOLD KRONECKER (1823—1891)

$$ds^2 = g_{ab} d\xi^a d\xi^b$$

$ds_o \rightarrow ds, \delta_{ab} \rightarrow g_{ab}$
 g_{ab} is metric tensor

How to determine metric tensor g_{ab} ?

Just express the distance (metric) ds on the surface in \mathbb{R}^3 !

Metric, metric tensor

CIARLET, P. G., GRATIE, L., and MARDARE, C. (2006)
 CIARLET, P. G. and LAURENT, F. (2003)
 CIARLET, P. G. (2005)

Mapping $\theta : \mathbb{R}^2 \ni \omega \rightarrow \mathbb{R}^3$

$$\mathbf{x}_o = \theta(\xi^a)$$

$$\mathbf{x} = \theta(\xi^a + d\xi^a)$$

vector algebra

$$(1) \quad d\mathbf{r} = \mathbf{g}_1 d\xi^1 + \mathbf{g}_2 d\xi^2$$

$$d\mathbf{r} = \mathbf{x} - \mathbf{x}_o$$

$$d\mathbf{r} = \theta(\xi^a + d\xi^a) - \theta(\xi^a)$$

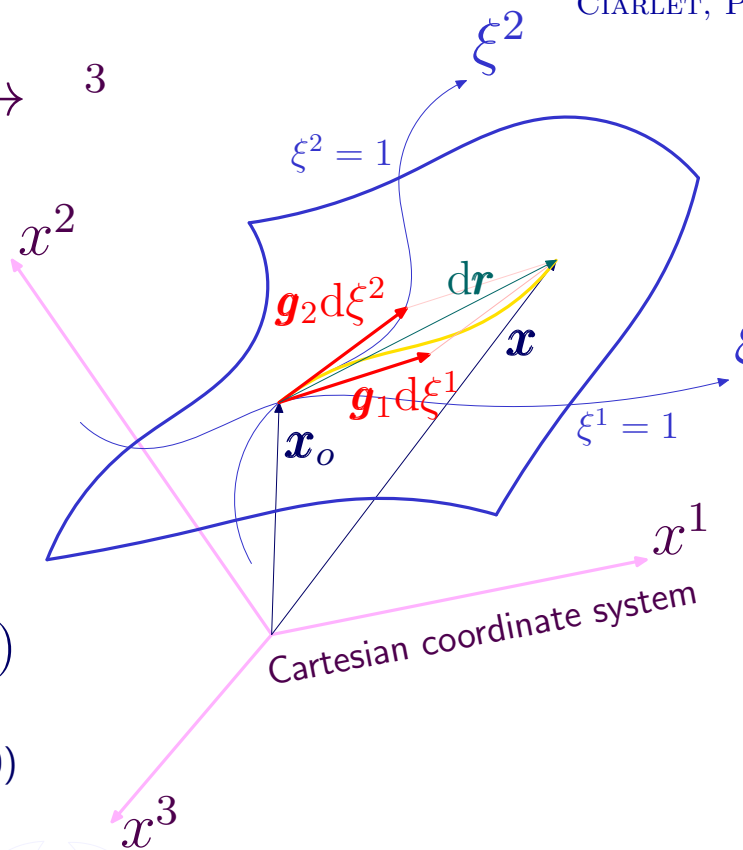
vector analysis (pro $d\xi^a \rightarrow 0$)

$$(2) \quad d\mathbf{r} = \frac{\partial \theta}{\partial \xi^1} d\xi^1 + \frac{\partial \theta}{\partial \xi^2} d\xi^2$$

$$ds^2 = d\mathbf{r} \cdot d\mathbf{r} = \left(\frac{\partial \theta}{\partial \xi^1} d\xi^1 \right)^2 + 2 \frac{\partial \theta}{\partial \xi^1} \frac{\partial \theta}{\partial \xi^2} d\xi^1 d\xi^2 + \left(\frac{\partial \theta}{\partial \xi^2} d\xi^2 \right)^2$$

Using Einstein summation

$$(3) \quad ds^2 = \frac{\partial \theta}{\partial \xi^a} \frac{\partial \theta}{\partial \xi^b} d\xi^a d\xi^b$$



Consequences:

$$(1) + (2): \quad \mathbf{g}_a = \frac{\partial \theta}{\partial \xi^a}$$

$$(3) + (4): \quad g_{ab} = \frac{\partial \theta}{\partial \xi^a} \frac{\partial \theta}{\partial \xi^b}$$

Metric tensor is symmetric

$$g_{ab} = g_{ba}$$

Base vectors are not orthogonal

$$\mathbf{g}_a \cdot \mathbf{g}_b = g_{ab} \neq \delta_{ab}$$

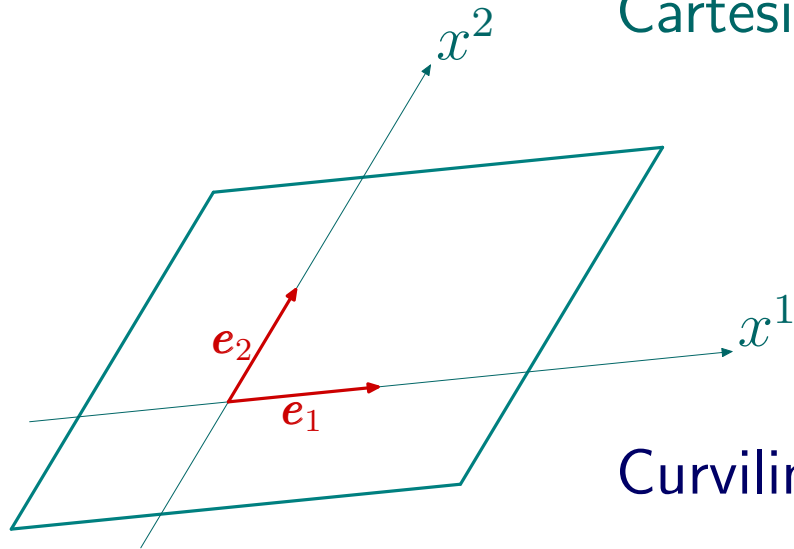
From the definition

$$ds^2 = g_{ab} d\xi^a d\xi^b \quad (4)$$

Abstract index notation

GREEN, A. E. and ZERNA, W. (1954)
 TABER, L. A. (2004)

Cartesian coordinate system (Euclidean space)



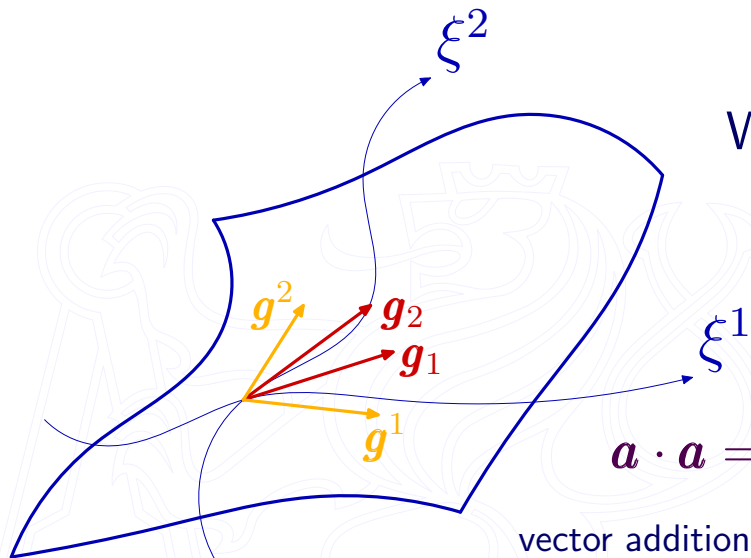
Base vectors are orthonormal

$$\mathbf{e}_a \cdot \mathbf{e}_b = \delta_{ab}$$

Vector is linear combination of base vectors

$$\mathbf{a} = a^1 \mathbf{e}_1 + a^2 \mathbf{e}_2 = a^a \mathbf{e}_a$$

Curvilinear coordinates (Non-euclidean space)



Base vectors are not (generally) orthonormal

$$\mathbf{g}_a \cdot \mathbf{g}_b = g_{ab} \neq \delta_{ab}$$

We are not used to it \Rightarrow We introduce new base vectors:

such that

$$\mathbf{g}^a \cdot \mathbf{g}_b = \delta_b^a$$

We may write

$$\mathbf{a} = a_a \mathbf{g}^a = a^a \mathbf{g}_a$$

e.g. scalar product

$$\mathbf{a} \cdot \mathbf{a} = (a_a \mathbf{g}^a) \cdot (a_b \mathbf{g}^b) = (a_a \mathbf{g}^a) \cdot (a^b \mathbf{g}_b) = a_a a^b \mathbf{g}^a \cdot \mathbf{g}_b = a_a a^a$$

vector addition

$$\mathbf{a} + \mathbf{b} = a_a \mathbf{g}^a + b_a \mathbf{g}^a = (a_a + b_a) \mathbf{g}^a$$

Tensor equation

$$\mathbf{T} = \mathbf{a} \otimes \mathbf{b} \Leftrightarrow T^{ab} \mathbf{g}_a \otimes \mathbf{g}_b = (a^a \mathbf{g}_a) \otimes (b^b \mathbf{g}_b) = a^a b^b \mathbf{g}_a \otimes \mathbf{g}_b$$

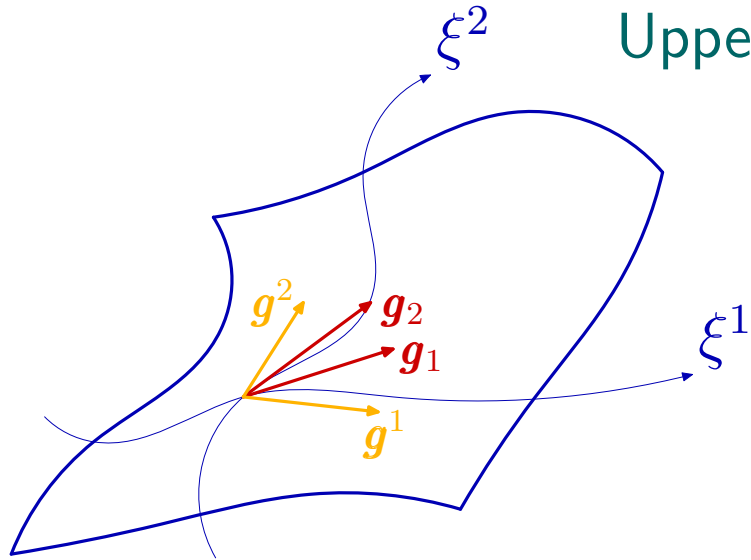
Leaving base vectors leads us to

Abstract index notation

$$a_a a^a = \mathbf{a} \cdot \mathbf{a}, \quad a^a + b^a \Leftrightarrow \mathbf{a} + \mathbf{b}, \quad a^a b^b \Leftrightarrow \mathbf{a} \otimes \mathbf{b}$$

Covariant derivative, transformation rules

SYNGE, J. L. and SCHILD, A. (1978)
LOVELOCK, D. and RUND, H. (1989)



Upper index

Definition: $g^{ab} = (g_{ab})^{-1}$

$$a_a g^a = a^a g_a$$

Multiplying by g_b leads at $a_a g^a \cdot g_b = a^a g_a \cdot g_b$

and $a_a \delta_b^a = a^a g_{ab}$

Multiply by g^{bd} and the definition $g^{bd} a_b = a^d$

Covariant derivative

$$\frac{\partial a}{\partial x^a} = \frac{\partial}{\partial x^a} (a^b g_b) = (\partial_a a^b) g_b + a^b \frac{\partial g_b}{\partial x^a}$$

We seek linear combination of the base vectors

$$\frac{\partial a}{\partial x^a} = (\partial_a a^b + \Gamma_{ac}^b a^c) g_b$$

Sign it as $\frac{\partial a}{\partial x^a} = \nabla_a a^b g_b$

$$\nabla_a a^b = \partial_a a^b + \Gamma_{ac}^b a^c$$

The Γ_{ab}^c (Christoffel symbol of the 2nd kind) exists:

$$\Gamma_{ab}^d = g^{dc} \frac{1}{2} (g_{ac,b} + g_{cb,a} - g_{ab,c})$$

Transformation rule for $x^a = x^a(\xi^b)$

Transformation rule for covectors

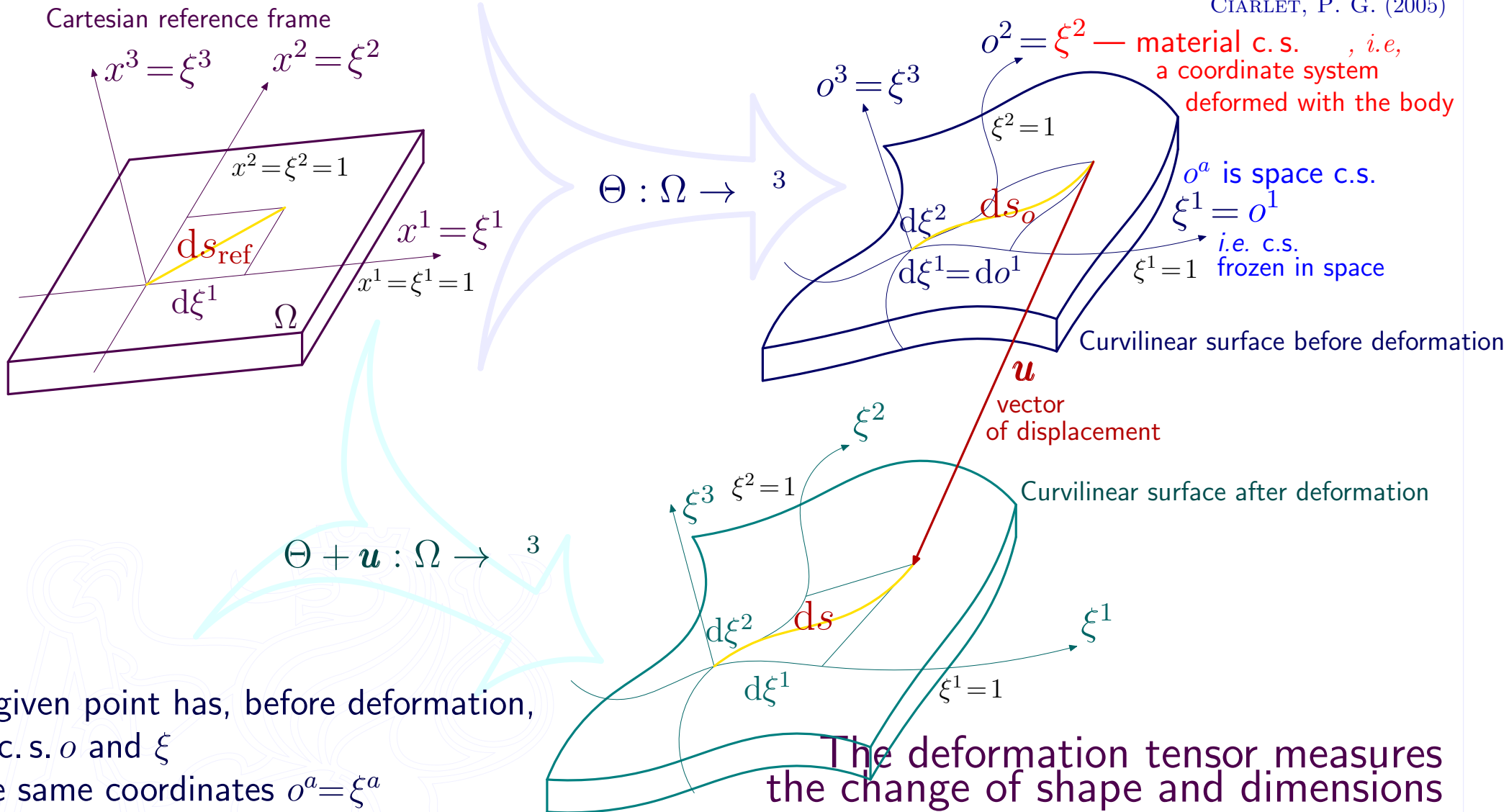
(covariant) $\frac{\partial \phi}{\partial x^a} = \frac{\partial \xi^b}{\partial x^a} \frac{\partial \phi}{\partial \xi^b}$

Transformation rule for vectors

(contravariant) $dx^a = \frac{\partial x^a}{\partial \xi^b} d\xi^b$

A deformation tensor

GREEN, A. E. AND ZERNA, W. (1954)
 ANTMAN, S. S. (2005)
 CIARLET, P. G. (2005)



A given point has, before deformation, in c.s. o and ξ the same coordinates $o^a = \xi^a$

The same point has, after deformation still the same coordinates at the c.s. ξ (ξ^a), but at the c.s. o , the coordinates are not the same and are given by (unknown) function $o^a = o^a(\xi^b)$

The shape and dimensions are described via coordinate systems

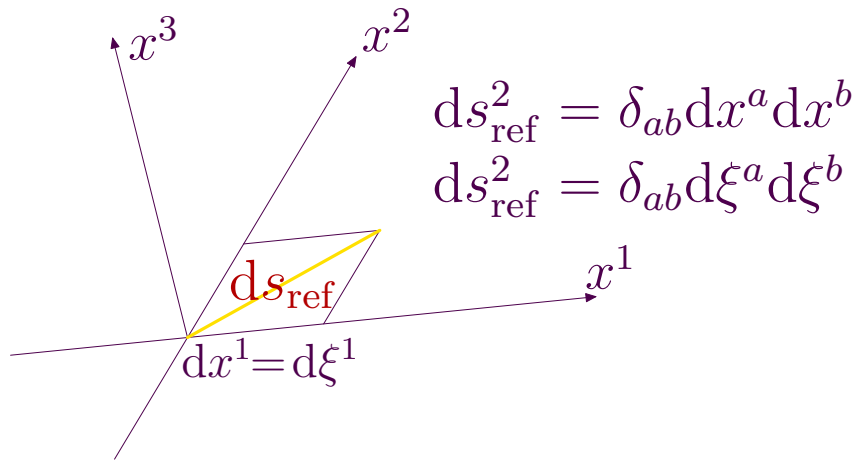
And they are of various types...

The distance of points on the surface

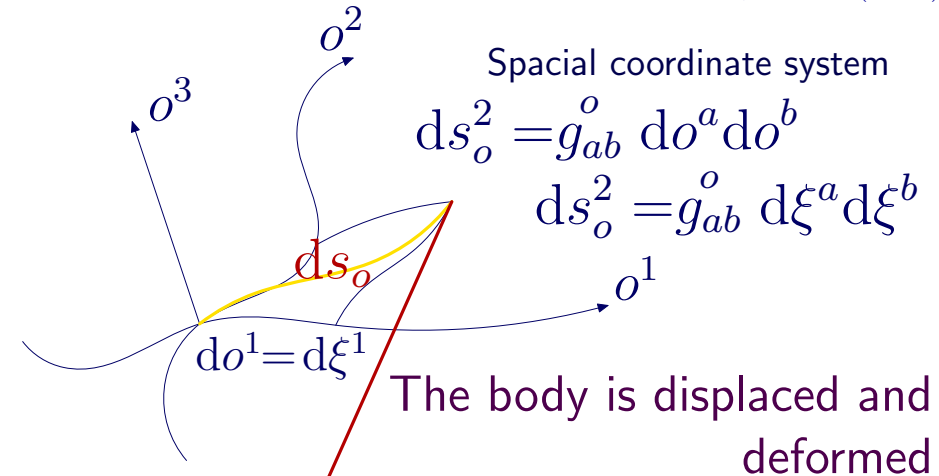
Deformation tensor

GREEN, A. E. AND ZERNA, W. (1954)
 ANTMAN, S. S. (2005)
 CIARLET, P. G. (2005)

Cartesian (spacial) coordinate system



Spacial coordinate system



Deformation is characterized by the change of a length

e.g. the element length, i.e. $(ds^2 - ds_o^2)$

$$ds^2 - ds_o^2 = (g_{ab}^\xi - g_{ab}^o) d\xi^a d\xi^b$$

The relation $ds^2 - ds_o^2 = 2 E_{ab} d\xi^a d\xi^b$ defines

Green-Lagrange-St. Venant deformation

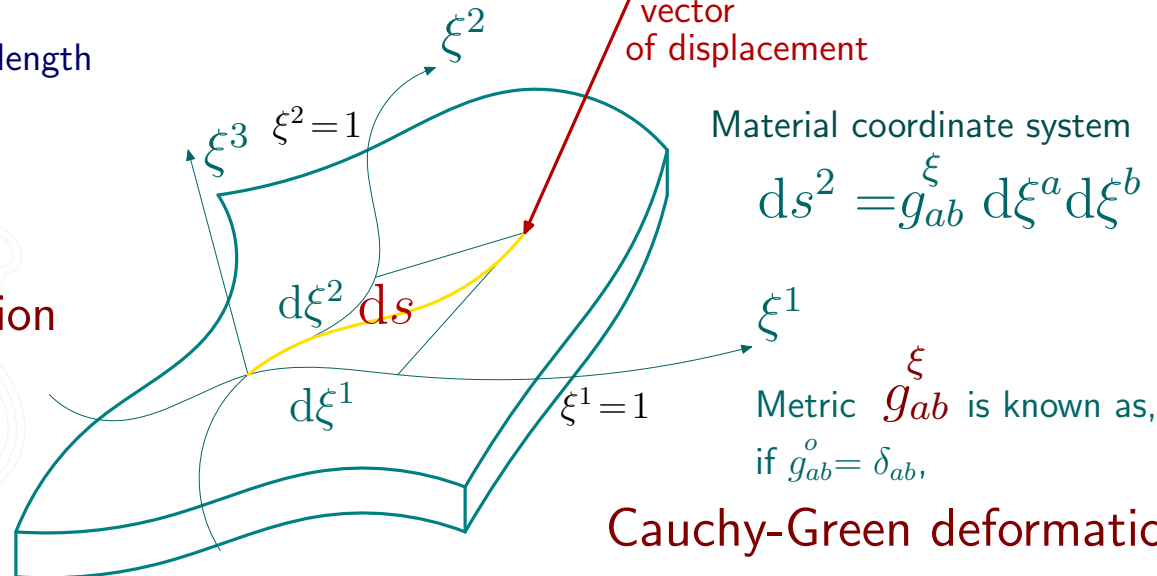
$$E_{ab}^\xi = \frac{1}{2} (g_{ab}^\xi - g_{ab}^o)$$

If o, ξ^o is Cartesian c. s.

i.e. $g_{ab}^o = \delta_{ab}$ and $o^a = \xi^a + u^a$

$$g_{ab}^\xi = \frac{\partial a^c}{\partial \xi^a} \frac{\partial a^d}{\partial \xi^b} g_{cd}^o = (\delta_a^c + \partial_a u^c)(\delta_b^d + \partial_b u^d) \delta_{cd} = \delta_{ab} + \partial_a u_b + \partial_b u_a + \partial_a u^c \partial_b u_c$$

$$E_{ab}^\xi = \frac{1}{2} (g_{ab}^\xi - \delta_{ab}) = \frac{1}{2} (\partial_a u_b + \partial_b u_a + \partial_a u^c \partial_b u_c) \text{ — the deformation tensor (lagrangian description)}$$



Cauchy-Green deformation

Euler deformation (Almans deformation): E_{ab}^x

Small deformation tensor and energy minimum principles

LOVE, A. E. H. (1927)

WASHIZU, K. (1975)

CIARLET, P. G. (2005)

Green-Lagrange-St. Venantova deformation **is linearized**

Small deformation tensor

$$E_{ab}^{\xi} = \frac{1}{2} (g_{ab}^{\xi} - g_{ab}^o) \Big|_{\text{in Cartesian coordinates}} = \frac{1}{2} (\partial_a u_b + \partial_b u_a + \cancel{\partial_a u^c \partial_b u_c})$$

$$\Rightarrow \varepsilon_{ab} = \frac{1}{2} (\partial_a u_b + \partial_b u_a)$$

passing into curvilinear coordinates

$$\partial_a \Rightarrow \nabla_a \quad (\text{WALD, R. M., 1984})$$

and check tensor transformation

Generally for small deformation

$$\varepsilon_{ab} = \frac{1}{2} (g_{ab}^{\xi} - g_{ab}^o) \Big|_{\text{lin.}} = \frac{1}{2} (\nabla_a u_b + \nabla_b u_a)$$

Principle of the total potential energy minimum \Rightarrow

The min principle of complementary energy

$$\hat{u}_a = \arg \min_{u_b \in} \Pi(u_c)$$

(MAUPERTUIS, 1746)

(EULER, 1744)

(LAGRANGE, 1788)

$$\hat{\sigma}_{ab} = \arg \min_{\sigma^{ab} \in} \Pi_c(\sigma^{ab})$$

The real state of a deformed body, \hat{u}_a , minimizes the total potential energy (on a set of admissible states,)

The equilibrium stress state, $\hat{\sigma}_{ab}$, minimizes the c. e. (on a set of admitted stress states)

$$\Pi(u_a) = a(u_a) - l(u_a)$$

$$= \{ \sigma^{ab} \mid \nabla_a \sigma^{ab} + p^b = 0 \text{ na } \Omega, \sigma^{ab} \ell_b = t^a \text{ na } \partial_t \Omega \}, \quad \Pi_c(\sigma^{ab}) = c(\sigma^{ab}) - l_u(\sigma^{ab})$$

The elastic strain energy

$$a(u_a) = \frac{1}{2} \int_{\Omega} E^{abcd} \varepsilon_{ab}(u_a) \varepsilon_{cd}(u_a) d\Omega$$

The complementary energy

$$c(\sigma^{ab}) = \frac{1}{2} \int_{\Omega} C_{abcd} \sigma^{ab} \sigma^{cd} d\Omega$$

The potential energy of the applied forces $p^a(\frac{N}{mm^3}), t^a(\frac{N}{mm^2})$

The work done through kinematic boundary conditions

$$l(u_a) = \int_{\Omega} p^a u_a d\Omega + \int_{\partial_t \Omega} t^a u_a d\Gamma$$

$$l_u(\sigma^{ab}) = \int_{\partial_u \Omega} \sigma^{ab} \tilde{u}_a \ell_b d\Gamma$$

Elasticity tensor E^{abcd} and compliance tensor C_{abcd}

CIARLET, P. G. (2005)

MAREŠ, T. (2006)

Isotropic material (λ, μ — Lamé coefficients)

$$E^{abcd} = \lambda g^{ab} g^{cd} + \mu g^{ac} g^{bd} + \mu g^{ad} g^{bc}$$

Orthotropic block

Young modulus in the direction ν^1

$$E_{11} = \frac{\sigma^{11}}{\varepsilon_1^1}$$

Poisson ratios

$$\nu_{12} = -\frac{\varepsilon_2^1}{\varepsilon_1^1}, \quad \nu_{13} = -\frac{\varepsilon_3^1}{\varepsilon_1^1}$$

Similarly in the direction of ν^2

$$E_{22} = \frac{\sigma^{22}}{\varepsilon_2^2}, \quad \nu_{21} = -\frac{\varepsilon_1^2}{\varepsilon_2^2}, \quad \nu_{23} = -\frac{\varepsilon_3^2}{\varepsilon_2^2}$$

and of ν^3

$$E_{33} = \frac{\sigma^{33}}{\varepsilon_3^3}, \quad \nu_{31} = -\frac{\varepsilon_1^3}{\varepsilon_3^3}, \quad \nu_{32} = -\frac{\varepsilon_2^3}{\varepsilon_3^3}$$

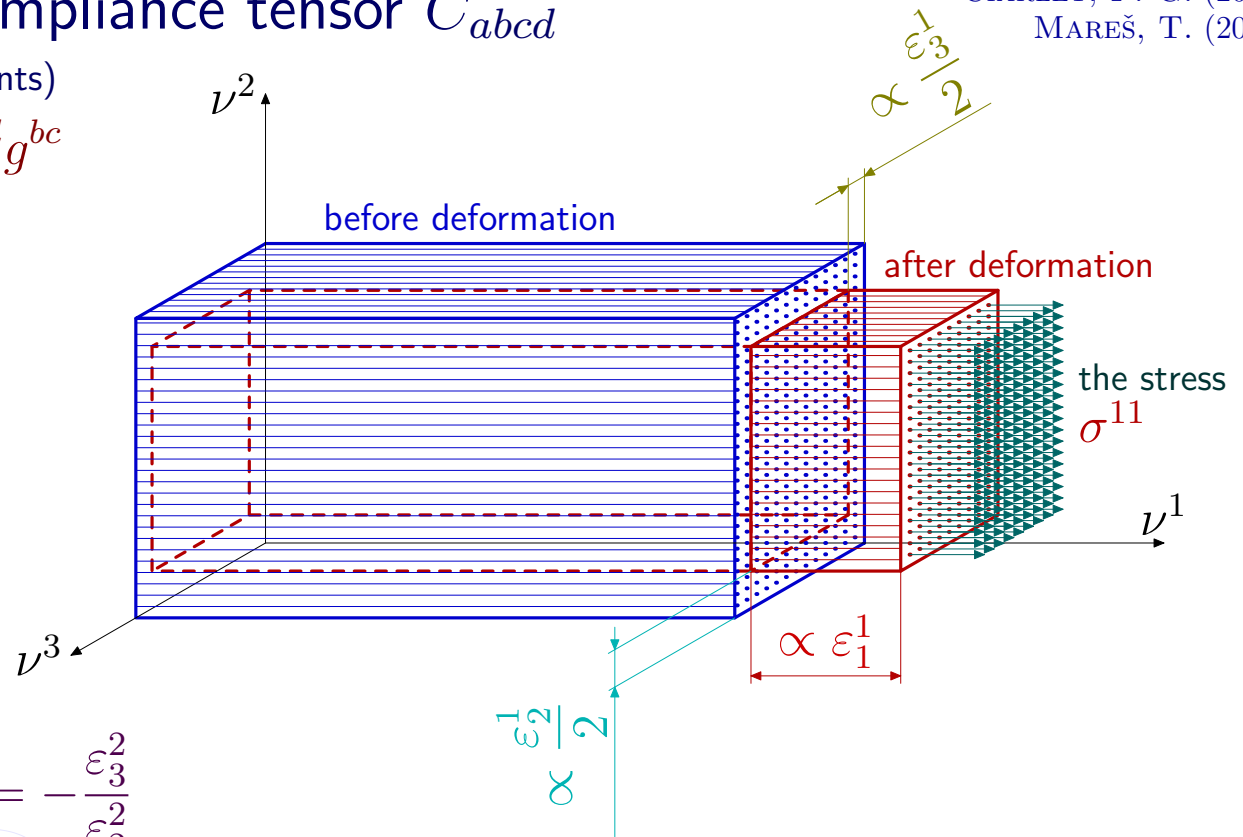
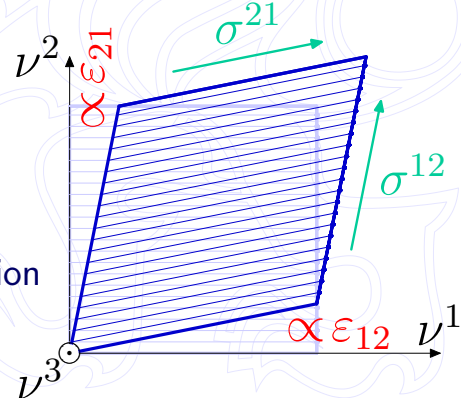
Pure shear

From the definition

$$\varepsilon_{12} = \varepsilon_{21}$$

the equilibrium equation

$$\sigma^{12} = \sigma^{21}$$



Strain in the ν^1 excited by all normal stresses

$$\varepsilon_{11} = \frac{\sigma^{11}}{E_{11}} - \nu_{21} \frac{\sigma^{22}}{E_{22}} - \nu_{31} \frac{\sigma^{33}}{E_{33}}$$

$$\varepsilon_{11} = \varepsilon_1^1 + \varepsilon_1^2 + \varepsilon_1^3$$

Similarly in the other directions (G_{23}, G_{31})

$$\sigma^{12} = \sigma^{21} = G_{12}(\varepsilon_{12} + \varepsilon_{21})$$

Compliance tensor C_{abcd}

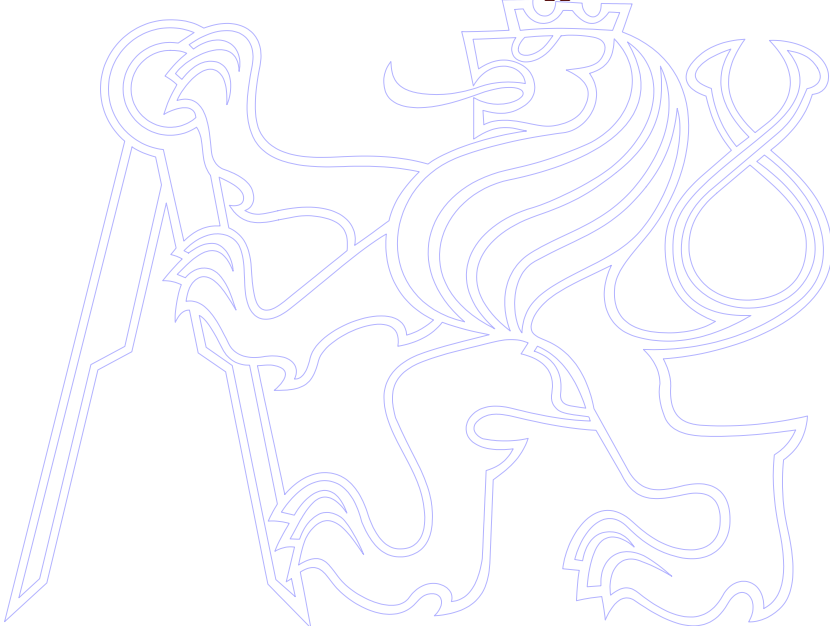
CIARLET, P. G. (2005)

MAREŠ, T. (2006)

in Cartesian coordinate system ν^a

aligned with the principal material axes of the orthotropic material

$$\begin{pmatrix} \varepsilon_{11} \\ \varepsilon_{12} \\ \varepsilon_{13} \\ \varepsilon_{21} \\ \varepsilon_{22} \\ \varepsilon_{23} \\ \varepsilon_{31} \\ \varepsilon_{32} \\ \varepsilon_{33} \end{pmatrix} = \begin{pmatrix} \frac{1}{E_{11}} & 0 & 0 & 0 & -\frac{\nu_{21}}{E_{22}} & 0 & 0 & 0 & -\frac{\nu_{31}}{E_{33}} \\ 0 & \frac{1}{4G_{12}} & 0 & \frac{1}{4G_{12}} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 \\ 0 & \frac{1}{4G_{12}} & 0 & \frac{1}{4G_{12}} & 0 & 0 & 0 & 0 & 0 \\ -\frac{\nu_{12}}{E_{11}} & 0 & 0 & 0 & \frac{1}{E_{22}} & 0 & 0 & 0 & -\frac{\nu_{32}}{E_{33}} \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{4G_{23}} & 0 & \frac{1}{4G_{23}} & 0 \\ 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{4G_{23}} & 0 & \frac{1}{4G_{23}} & 0 \\ -\frac{\nu_{13}}{E_{11}} & 0 & 0 & 0 & -\frac{\nu_{23}}{E_{22}} & 0 & 0 & 0 & \frac{1}{E_{33}} \end{pmatrix} \begin{pmatrix} \sigma_{11} \\ \sigma_{12} \\ \sigma_{13} \\ \sigma_{21} \\ \sigma_{22} \\ \sigma_{23} \\ \sigma_{31} \\ \sigma_{32} \\ \sigma_{33} \end{pmatrix}$$



$$\varepsilon_{ab}^{\nu} = C_{abcd}^{\nu} \sigma_{cd}^{\nu}$$

$$C_{abcd} = C_{cdab} = C_{bacd}$$

Energy ↗

↖ Equilibrium

Elasticity tensor E^{abcd}

in Cartesian coordinate system ν^a

by inversion of the previous expression

CIARLET, P. G. (2005)

MAREŠ, T. (2006)

$$\sigma^{ij} = E^{ijkl} \varepsilon_{kl}$$

$$\left\{ E^{ijkl} \right\}_{\{ij\{kl\}} = \begin{pmatrix} \Phi_{1111} & 0 & 0 & 0 & \Phi_{1122} & 0 & 0 & 0 & \Phi_{1133} \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ \Phi_{2211} & 0 & 0 & 0 & \Phi_{2222} & 0 & 0 & 0 & \Phi_{2233} \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ \Phi_{3311} & 0 & 0 & 0 & \Phi_{3322} & 0 & 0 & 0 & \Phi_{3333} \end{pmatrix}$$

$$E^{abcd} = E^{bacd}$$

$$\Phi_{1111} = \frac{1 - \nu_{23}\nu_{32}}{N} E_{11}, \quad \Phi_{1122} = \frac{\nu_{21} + \nu_{23}\nu_{31}}{N} E_{11}, \quad \Phi_{1133} = \frac{\nu_{31} + \nu_{32}\nu_{21}}{N} E_{11}$$

$$\Phi_{2211} = \frac{\nu_{12} + \nu_{13}\nu_{32}}{N} E_{22}, \quad \Phi_{2222} = \frac{1 - \nu_{13}\nu_{31}}{N} E_{22}, \quad \Phi_{2233} = \frac{\nu_{32} + \nu_{31}\nu_{12}}{N} E_{22}$$

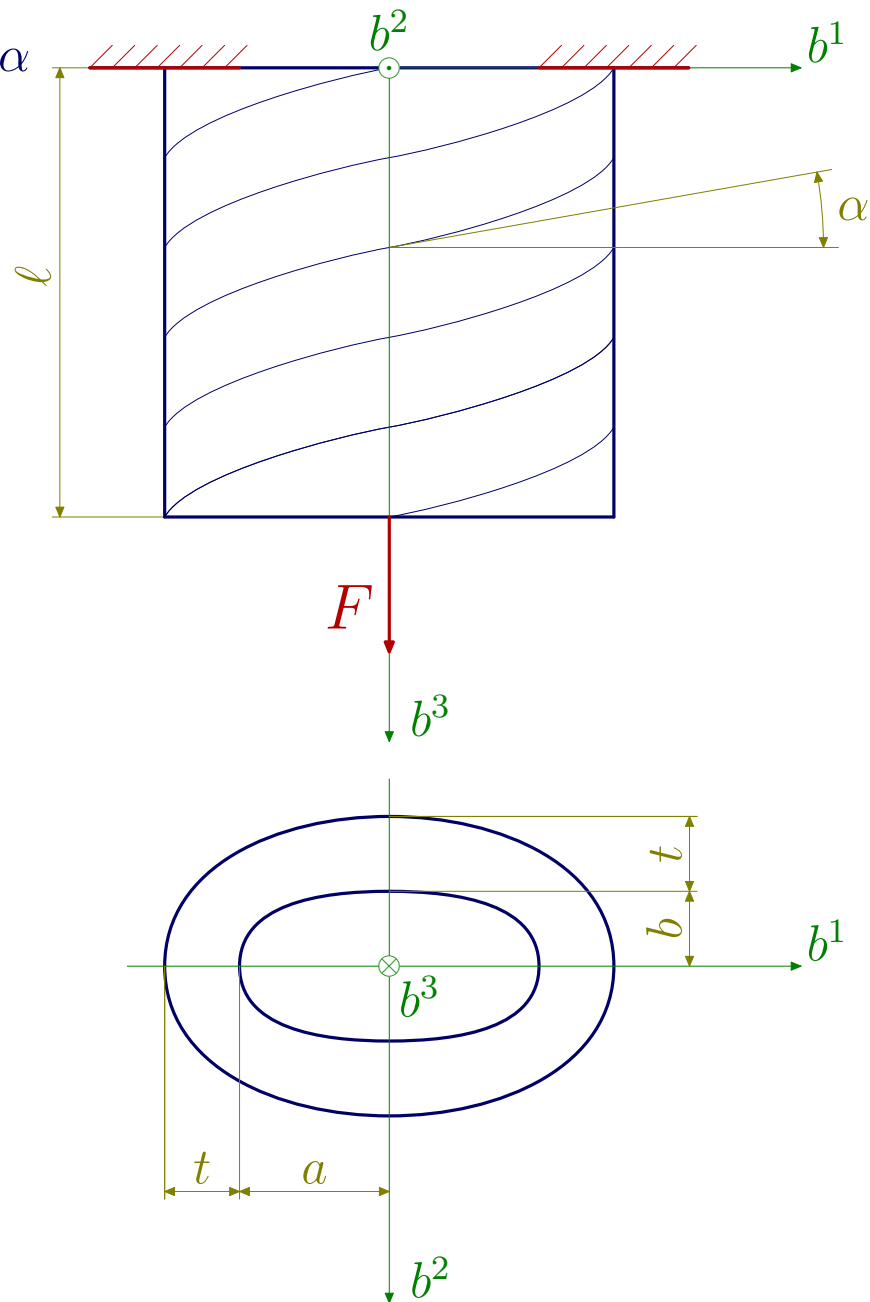
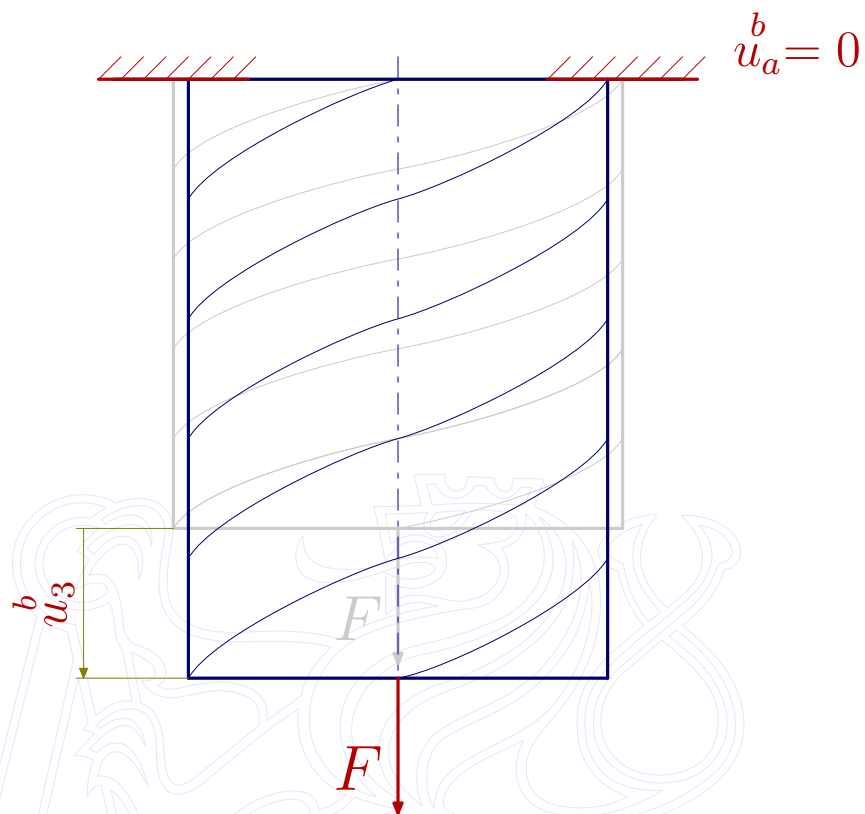
$$\Phi_{3311} = \frac{\nu_{13} + \nu_{12}\nu_{23}}{N} E_{33}, \quad \Phi_{3322} = \frac{\nu_{23} + \nu_{21}\nu_{13}}{N} E_{33}, \quad \Phi_{3333} = \frac{1 - \nu_{12}\nu_{21}}{N} E_{33}$$

$$N = 1 - \nu_{12}\nu_{21} - \nu_{23}\nu_{32} - \nu_{31}\nu_{13} - \nu_{12}\nu_{23}\nu_{31} - \nu_{13}\nu_{32}\nu_{21}$$

$$\text{Energy } (E^{abcd} = E^{cdab}) \Rightarrow \Phi_{1122} = \Phi_{2211} \Rightarrow \nu_{21} E_{11} = \nu_{12} E_{22}, \text{ etc.}$$

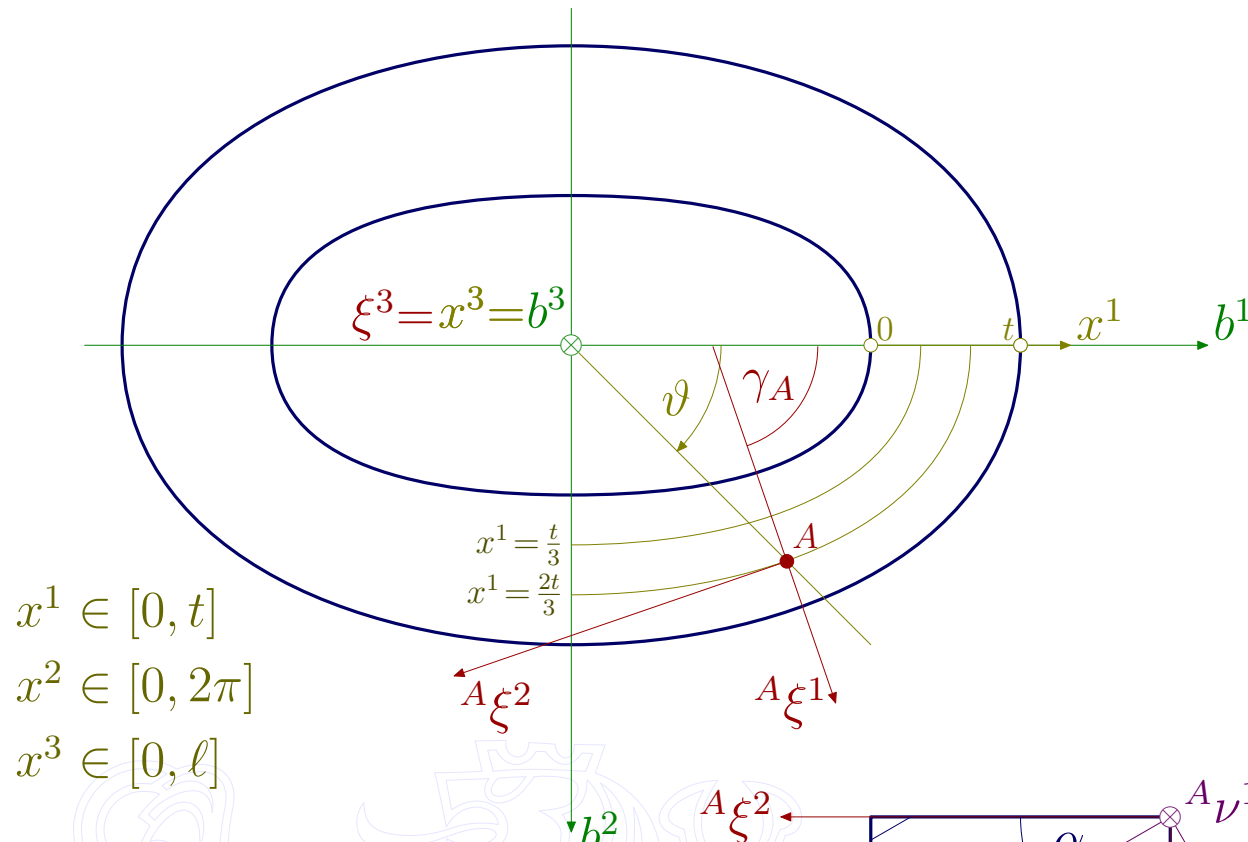
The problem

The thick-walled elliptic tube coiled with an angle α loaded with Force F and clamped as seen at Fig.



Used coordinate systems

- b^a — Cartesian coordinate system
- x^a — Elliptic coordinate system
- ξ^a — Local Cartesian coordinate s.
- ν^a — Main c. s. of the local orthotropy



$$x^1 \in [0, t]$$

$$x^2 \in [0, 2\pi]$$

$$x^3 \in [0, \ell]$$

$$b^1 = (a + x^1) \cos x^2$$

$$b^2 = (b + x^1) \sin x^2$$

$$b^3 = x^3$$

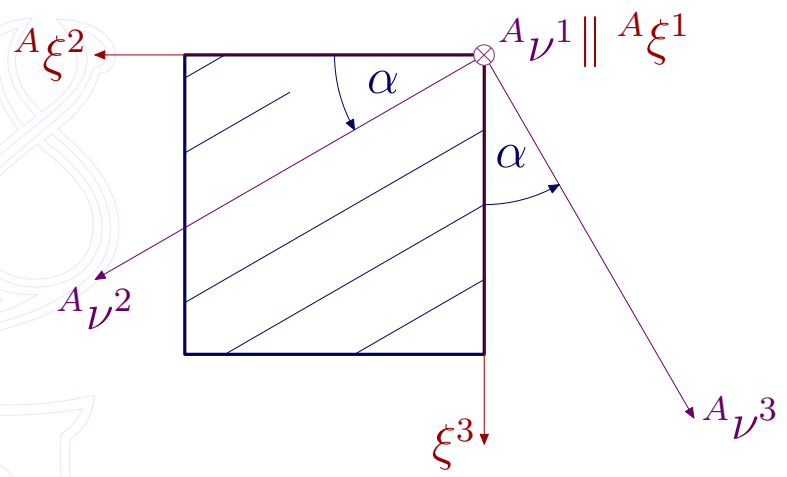
$$\vartheta = \vartheta(x^1, x^2)$$

$$\gamma = \gamma(x^1, x^2)$$

$$g_{ab}^b = \delta_{ab}$$

$$g_{ab}^\xi = \delta_{ab}$$

$$g_{ab}^\nu = \delta_{ab}$$



Metric tensor for integration

$$g_{ab}^x = \frac{\partial b^c}{\partial x^a} \frac{\partial b^d}{\partial x^b} \delta_{cd} \quad \frac{\partial b^a}{\partial x^b} = \begin{pmatrix} \cos x^2 & -(a + x^1) \sin x^2 & 0 \\ \sin x^2 & (b + x^1) \cos x^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}_{a \uparrow b}$$

$$g_{ab}^x = \begin{pmatrix} 1 & (b - a) \sin x^2 \cos x^2 & 0 \\ (b - a) \sin x^2 \cos x^2 & (a + x^1)^2 \sin^2 x^2 + (b + x^1)^2 \cos^2 x^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

Elasticity tensor transformation

$$E^{abcd}_x = \frac{\partial x^a}{\partial \nu^i} \frac{\partial x^b}{\partial \nu^j} \frac{\partial x^c}{\partial \nu^k} \frac{\partial x^d}{\partial \nu^l} E^{ijkl}_\nu$$

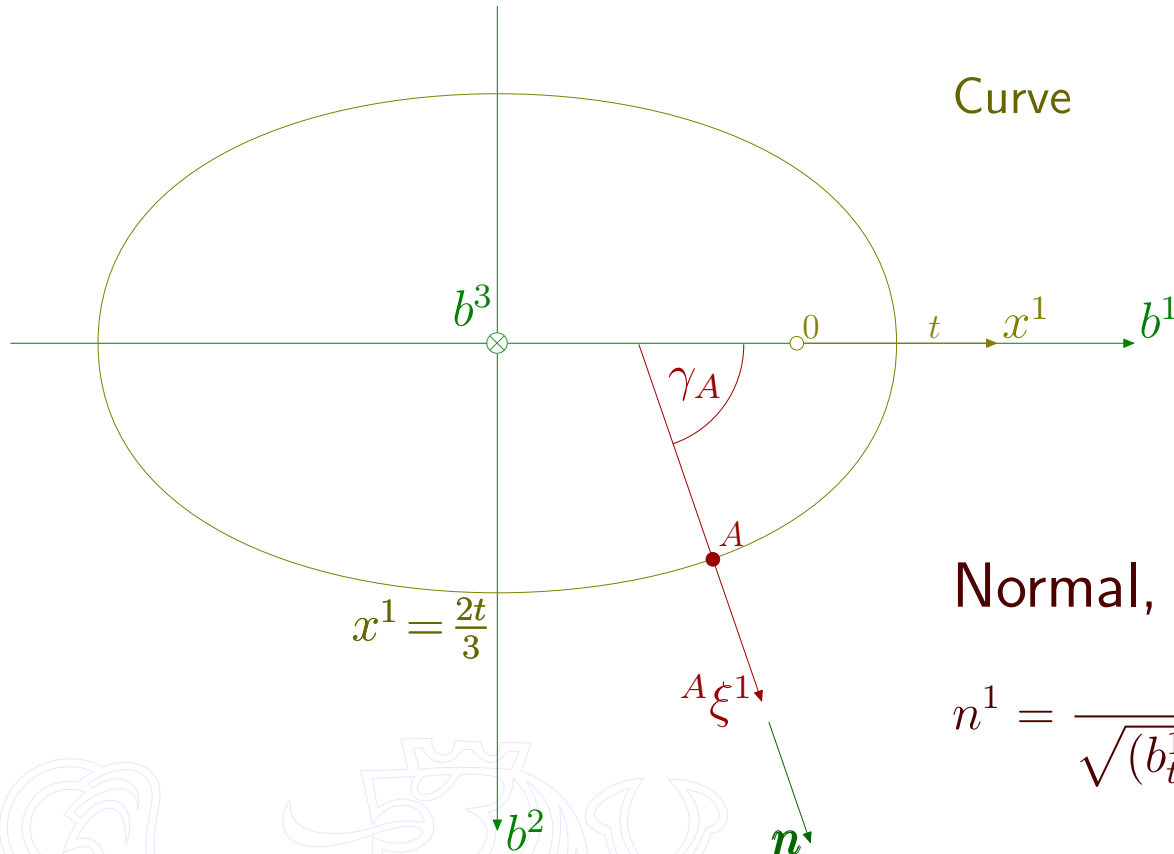
$$\frac{\partial x^a}{\partial \nu^b} = \frac{\partial x^a}{\partial b^c} \frac{\partial b^c}{\partial \xi^d} \frac{\partial \xi^d}{\partial \nu^b} \quad \frac{\partial x^a}{\partial b^b} = \left(\frac{\partial b^a}{\partial x^b} \right)^{-1} =$$

$$= \frac{1}{a \sin^2 x^2 + b \cos^2 x^2 + x^1} \begin{pmatrix} (b + x^1) \cos x^2 & (a + x^1) \sin x^2 & 0 \\ -\sin x^2 & \cos x^2 & 0 \\ 0 & 0 & a \sin^2 x^2 + b \cos^2 x^2 + x^1 \end{pmatrix}$$

$$\frac{\partial b^a}{\partial \xi^b} = \begin{pmatrix} \cos \gamma_A & -\sin \gamma_A & 0 \\ \sin \gamma_A & \cos \gamma_A & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

$$\frac{\partial \xi^a}{\partial \nu^b} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \cos \alpha & -\sin \alpha \\ 0 & \sin \alpha & \cos \alpha \end{pmatrix}$$

Normal to the ellipse $\Rightarrow \gamma_A$



Curve

$$b^1 = (a + x^1) \cos x^2$$

$$b^2 = (b + x^1) \sin x^2$$

$$\cos \gamma_A = (1, 0) \cdot \mathbf{n}$$

$$\sin \gamma_A = \pm |(1, 0) \times \mathbf{n}|$$

Normal, \mathbf{n}

$$t = \frac{\partial}{\partial x^2}$$

$$n^1 = \frac{b_t^2}{\sqrt{(b_t^1)^2 + (b_t^2)^2}}, \quad n^2 = \frac{-b_t^1}{\sqrt{(b_t^1)^2 + (b_t^2)^2}}$$

$$\mathbf{n} = \frac{1}{d} \begin{pmatrix} (b + x^1) \cos x^2 \\ (a + x^1) \sin x^2 \end{pmatrix}$$

$$d = \sqrt{(a + x^1)^2 \sin^2 x^2 + (b + x^1)^2 \cos^2 x^2}$$

Angle γ_A consequently

$$\cos \gamma_A = \frac{b + x^1}{d} \cos x^2$$

$$\sin \gamma_A = \frac{a + x^1}{d} \sin x^2$$

Total potential energy of the tube

SYNGE, J. L. and SCHILD, A. (1978)

LOVELOCK, D. and RUND, H. (1989)

CIARLET, P. G. (2005)

GNU MAXIMA gamma.mac

Principle of the total potential energy minimum

$$\hat{u}_a = \arg \min_{u_b \in \mathbb{U}} \Pi(u_c)$$

$$\varepsilon_{ab} = \frac{1}{2} (\nabla_a u_b + \nabla_b u_a)$$

The real state of a deformed body minimizes the total potential energy
(on a set of admissible states, \mathbb{U})

$$\nabla_a u_b = \partial_a u_b - \Gamma_{ab}^c u_c$$

$$\Pi(u_a) = a(u_a) - l(u_a)$$

Christoffel symbol of the 2nd kind

The elastic strain energy

$$\Gamma_{ab}^d = g^{dc} \frac{1}{2} (g_{ac,b} + g_{cb,a} - g_{ab,c})$$

$$a(u_a) = \frac{1}{2} \int_{\Omega} E^{abcd} \varepsilon_{ab}(u_a) \varepsilon_{cd}(u_a) d\Omega$$

In c. s. x :

$$\Gamma_{12}^1 = \Gamma_{21}^1 = \frac{(a-b) \cos x^2 \sin x^2}{(b-a) \cos^2 x^2 + x^1 + a}$$

The potential energy of the applied forces $p^a(\frac{N}{mm^3}), t^a(\frac{N}{mm^2})$

$$l(u_a) = \int_{\Omega} p^a u_a d\Omega + \int_{\partial_t \Omega} t^a u_a d\Gamma$$

$$\Gamma_{12}^2 = \Gamma_{21}^2 = \frac{1}{(b-a) \cos^2 x^2 + x^1 + a}$$

$$\varepsilon_{ab}^x = \frac{1}{2} (\partial_a^x u_b^x + \partial_b^x u_a^x - 2 \Gamma_{ab}^c u_c^x)$$

$$\Gamma_{22}^1 = -\frac{(x^1)^2 + x^1(a+b) + ab}{(b-a) \cos^2 x^2 + x^1 + a}$$

$$\Gamma_{ab}^c u_c^x = \Gamma_{ab}^1 u_1^x + \Gamma_{ab}^2 u_2^x + \Gamma_{ab}^3 u_3^x$$

$$\Gamma_{22}^2 = \frac{(a-b) \cos x^2 \sin x^2}{(b-a) \cos^2 x^2 + x^1 + a}$$

$$\Gamma_{ab}^1 = \frac{1}{J} \begin{pmatrix} 0 & (a-b) \cos x^2 \sin x^2 & 0 \\ (a-b) \cos x^2 \sin x^2 & -((x^1)^2 + x^1(a+b) + ab) & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\Gamma_{ab}^2 = \frac{1}{J} \begin{pmatrix} 0 & 1 & 0 \\ 1 & (a-b) \cos x^2 \sin x^2 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \Gamma_{ab}^3 = 0$$

$$J = (b-a) \cos^2 x^2 + x^1 + a$$

The solution is sought in the form of Fourier series

Boundary condition

$$x^3 = 0 : \begin{matrix} x \\ u_1 = 0 \\ x \\ u_2 = 0, u_3 = 0 \end{matrix}$$

$$x u_1 = \sum_{j,k,m=-K}^K a_1^{jkm} x^3 e^{i(jx^1 \frac{2\pi}{t} + kx^2 + mx^3 \frac{2\pi}{\ell})} \quad K = \infty \quad (3)$$

The potential energy of the F

$$l(u_a) = \int_S \frac{F}{S} x u_3 dS \quad x u_2 = \sum_{j,k,m=-K}^K a_2^{jkm} x^3 e^{i(jx^1 \frac{2\pi}{t} + kx^2 + mx^3 \frac{2\pi}{\ell})} \quad x u_3 = \sum a_3 \varphi$$

$\min_{BC \text{ fulfilled}} (a - l)$

Sign $x u_{1,2} = \sum a_{1,2} \varphi$ ($\varphi = x^3 \phi$), pak

$$\frac{\partial a}{\partial a_1^{jkm}} = 0, \quad \frac{\partial(a - l)}{\partial \bar{u}} = 0$$

$$\frac{\partial x u_a}{\partial x^b} = \begin{pmatrix} \sum a_1 \varphi ij \frac{2\pi}{t} & \sum a_1 \varphi ik & \sum a_1 (\varphi im \frac{2\pi}{\ell} + \phi) \\ \sum a_2 \varphi ij \frac{2\pi}{t} & \sum a_2 \varphi ik & \sum a_2 (\varphi im \frac{2\pi}{\ell} + \phi) \\ \sum a_3 \varphi ij \frac{2\pi}{t} & \sum a_3 \varphi ik & \sum a_3 (\varphi im \frac{2\pi}{\ell} + \phi) \end{pmatrix}$$

$$a(u_a) = \frac{1}{2} \int_{\Omega} E^{abcd} \varepsilon_{ab}(u_a) \varepsilon_{cd}(u_a) d\Omega$$

Transformation

$$E^{abcd} = \frac{\partial x^a}{\partial \nu^i} \frac{\partial x^b}{\partial \nu^j} \frac{\partial x^c}{\partial \nu^k} \frac{\partial x^d}{\partial \nu^l} E^{ijkl}$$

$$E^{abcd} = E^{bacd} \Rightarrow \varepsilon_{ab} \rightarrow \partial_a x u_b - \Gamma_{ab}^c x u_c$$

is performed in GNU OCTAVE syntax simply

$$a = \frac{1}{2} \int_{\Omega} \left(\partial_a x u_b - \Gamma_{ab}^p x u_p \right) E^{abcd} \left(\partial_c x u_d - \Gamma_{cd}^p x u_p \right) \left| g_{ab} \right|^{\frac{1}{2}} d^3x$$

`xnu=xb*bxixinu`

`Ex=kron(xnu,xnu)*Enu*kron(xnu',xnu')`

Derivatives

$$\frac{\partial l}{\partial \bar{u}} = F \frac{\partial a}{\partial u, a_{1,2}} = ?$$

We have chosen

$$u_{1,2}^x = \sum_{j,k,m=-K}^K a_{1,2}^{jkm} \varphi^{jkm}$$

$$\varphi^{jkm} = x^3 e^{ijx^1 \frac{2\pi}{t}} \cdot e^{ikx^2} \cdot e^{imx^3 \frac{2\pi}{\ell}}$$

GNU OCTAVE

j=(-3:1:3); k=(-3:1:3); m=(-3:1:3);

$$\begin{pmatrix} u_1 \\ u_2 \\ u_3 \end{pmatrix} = ux = [\text{phi}, \text{zeros}(1, 686); \text{zeros}(1, 343), \text{phi}, \text{zeros}(1, 343); \text{zeros}(1, 686), \text{phi}] * A$$

A is a vector of coefficients

$$\left\{ \frac{\partial u_a}{\partial x^b} \right\}_{ab} = B * A$$

```
B=[i*2*pi/t*phi.*kron(kron(j, jedna), jedna), zeros(1, 343), zeros(1, 343);
i*phi.*kron(kron(jedna, k), jedna), zeros(1, 343), zeros(1, 343);
i*2*pi/ell*phi.*kron(kron(jedna, jedna), m)+phi*ones(1, 343), zeros(1, 686);
zeros(1, 343), i*2*pi/t*phi.*kron(kron(j, jedna), jedna), zeros(1, 343);
zeros(1, 343), i*phi.*kron(kron(jedna, m), jedna), zeros(1, 343);
zeros(1, 343), i*2*pi/ell*phi.*kron(kron(je, je), m)+phi*ones(1, 343), zeros(1, 343);
zeros(1, 343), zeros(1, 343), i*2*pi/t*phi.*kron(kron(j, jedna), jedna);
zeros(1, 343), zeros(1, 343), i*phi.*kron(kron(jedna, k), jedna);
zeros(1, 686), i*2*pi/ell*phi.*kron(kron(jedna, jedna), m)+phi*ones(1, 343)]
```

Integration of the elastic energy

$$\Gamma_{ab}^c \dot{u}_c^x = \Gamma_{ab}^1 \dot{u}_1^x + \Gamma_{ab}^2 \dot{u}_2^x + \Gamma_{ab}^3 \dot{u}_3^x$$

$$\left\{ \Gamma_{ab}^p \dot{u}_p^x \right\}_{ab} = \left\{ \Gamma_{ab}^1 \right\}_{ab} * [\text{phi}, \text{zeros}(1, 686)] * A + \\ + \left\{ \Gamma_{ab}^2 \right\}_{ab} * [\text{zeros}(1, 343), \text{phi}, \text{zeros}(1, 343)] * A$$

$$\left(\partial_a \dot{u}_b^x - \Gamma_{ab}^p \dot{u}_p^x \right) = (B - \text{Gam}) * A$$

$$J = (b - a) * (\cos(x_2)) ** 2 + x_1 + a$$

$$G1 = 1/J * [0, (a - b) * \cos(x_2) * \sin(x_2), 0;$$

$$(a - b) * \cos(x_2) * \sin(x_2), -((x_1) ** 2 + x_1 * (a + b) + a * b), 0; 0, 0, 0]$$

$$G2 = 1/J * [0, 1, 0; 1, (a - b) * \cos(x_2) * \sin(x_2), 0; 0, 0, 0]$$

$$\text{Gam} = \text{vec}(G1') * [\text{phi}, \text{zeros}(1, 686)] + \text{vec}(G2') * [\text{zeros}(1, 343), \text{phi}, \text{zeros}(1, 343)]$$

Elasticity energy

$$a = \frac{1}{2} A^T K A$$

Stiffness matrix

$$K = \int_0^\ell \int_0^{2\pi} \int_0^t (B - \text{Gam})' * E_x * (B - \text{Gam}) * \text{sqrt}(\det(gx)) dx^1 dx^2 dx^3 \\ gx = (xb ** (-1))' * xb ** (-1)$$

Integrand is expressed in GNU OCTAVE, integrate it numerically (energy.m)

The equation, right hand side and solution

$$\frac{\partial a}{\partial A} = \frac{\partial l}{\partial A} \quad \frac{\partial a}{\partial A} = KA \quad KA = P$$

$$l = \int_S \frac{F}{S} u_3 \, dS$$

$$l = \int_0^{2\pi} \int_0^t \frac{F}{S} [\text{zeros}(1,343), \text{zeros}(1,343), \text{phi}] * \\ * \text{sqrt}(\det(\text{gx})) \, dx^1 dx^2 * A$$

$$P = \frac{\partial l}{\partial A} = \begin{pmatrix} \text{zeros}(363) \\ \text{zeros}(363) \\ \int_0^{2\pi} \int_0^t \frac{F}{S} \text{phi}' * \text{sqrt}(\det(\text{gx})) \, dx^1 dx^2 \end{pmatrix}$$

x1=...; x2=...; x3=...

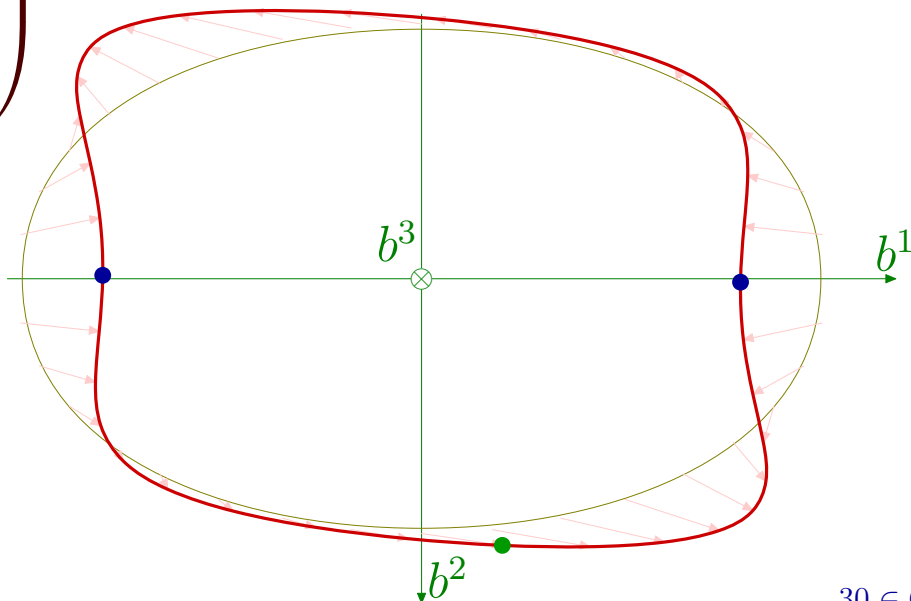
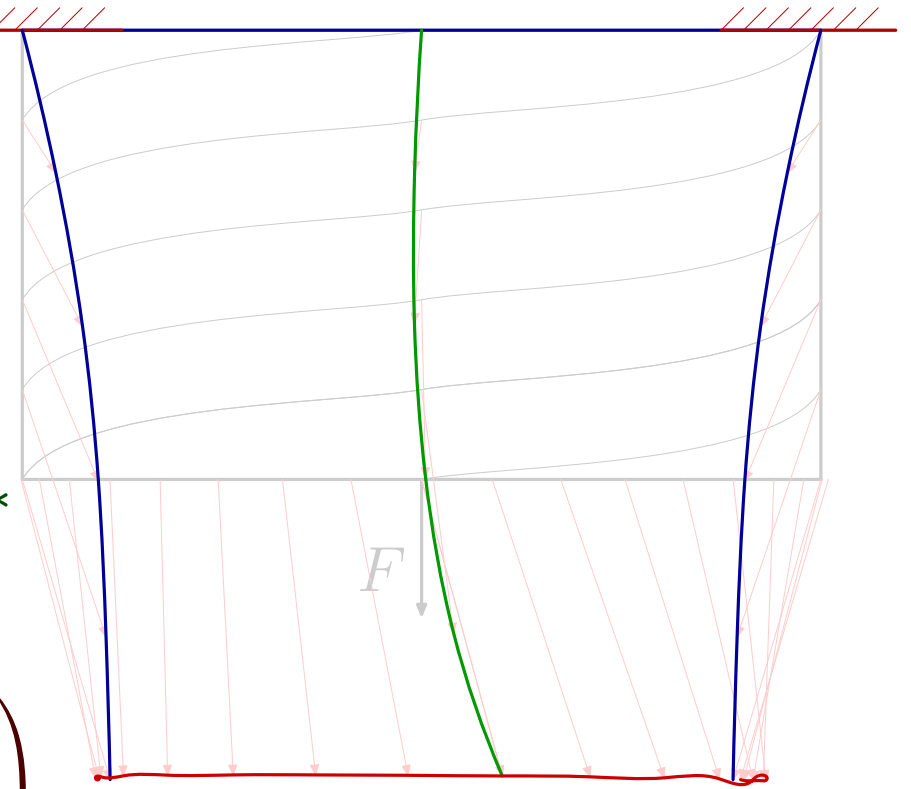
A=K*(-1)*P

phi=x3*kron(kron(exp(i*j*x1*2*pi/t), ...

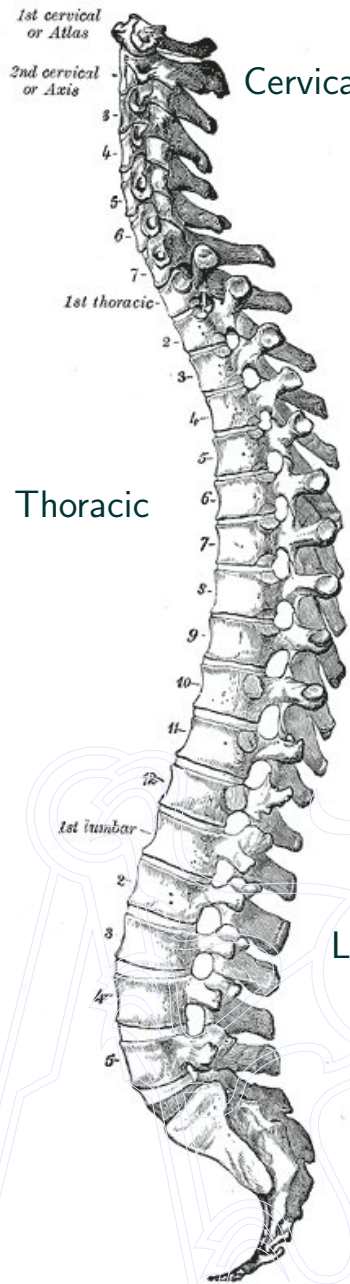
ux=real([phi,zeros(1,siz),zeros(1,...

xb=1/(a*(sin(x2))**2+b*(cos(x2))**2+...

ub=xb*ux



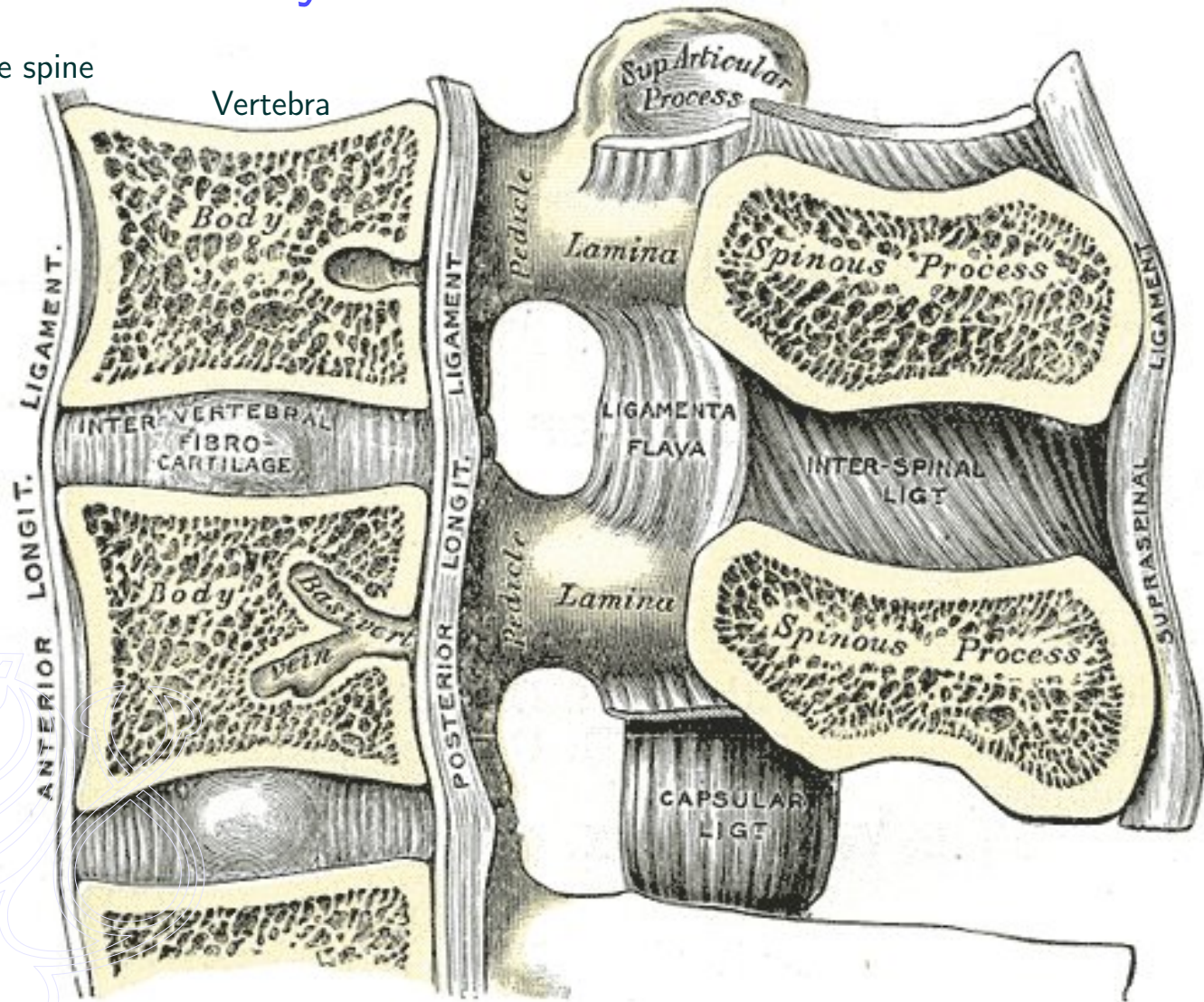
Deformation analysis of the Intervertebral disk



Cervical part of the spine

Thoracic

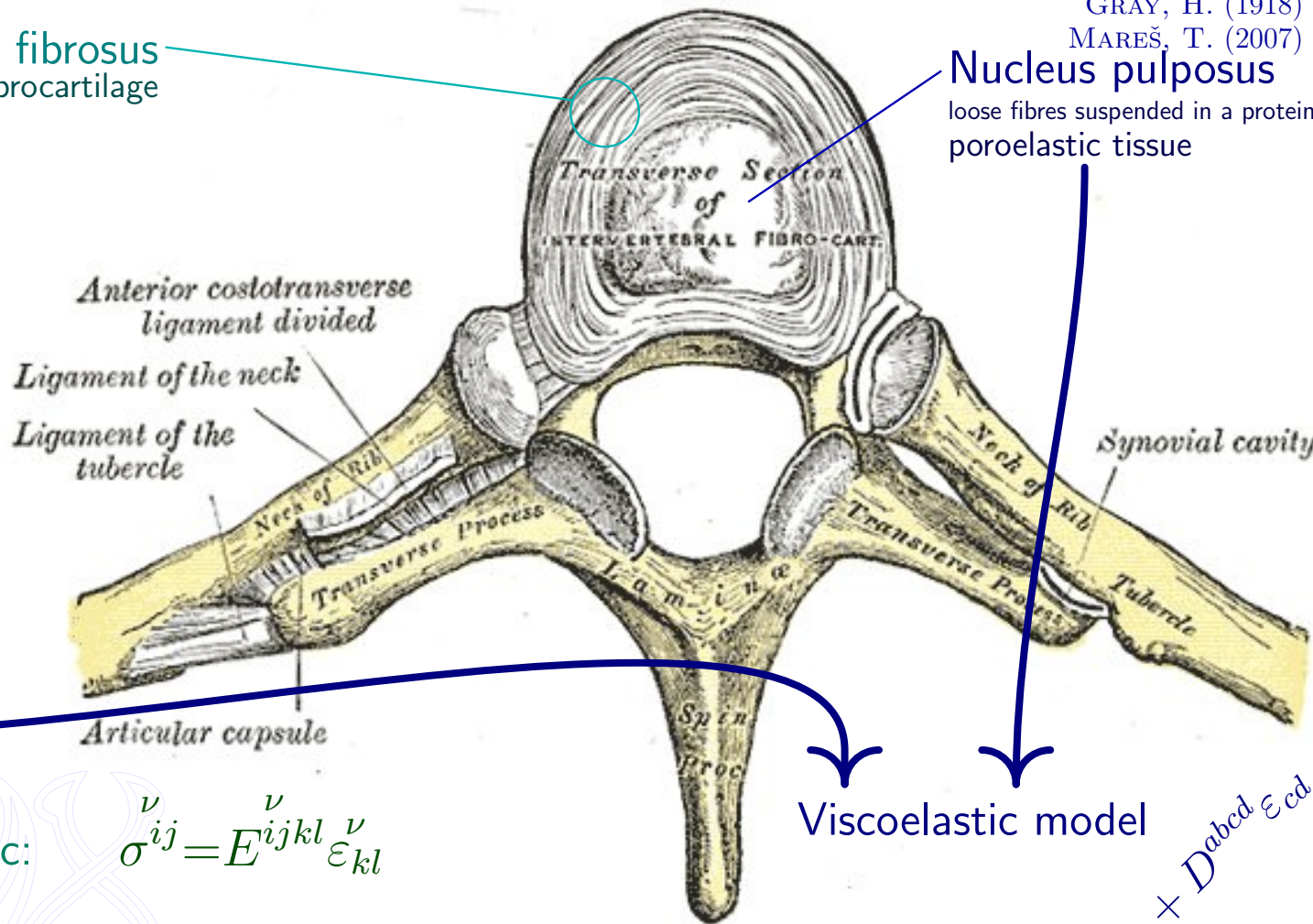
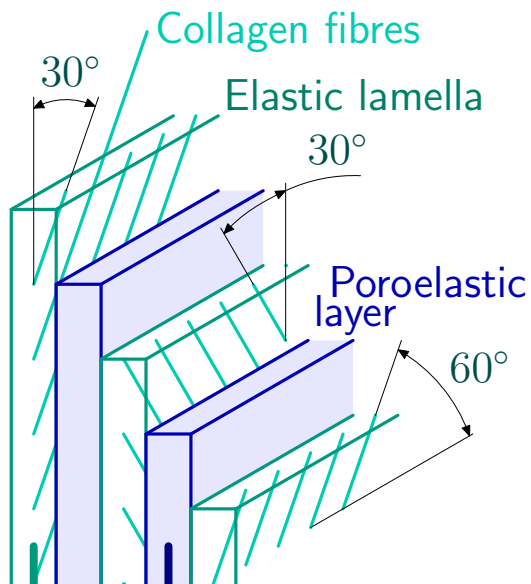
Lumbar



Transversal section

Annulus fibrosus
layers of fibrocartilage

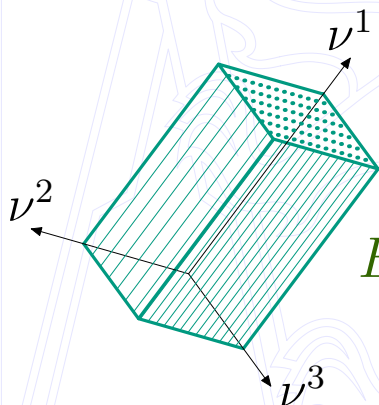
GRAY, H. (1918)
MAREŠ, T. (2007)
Nucleus pulposus
loose fibres suspended in a protein gel
poroelastic tissue



Viscoelastic model

Elastic locally orthotropic:

$$\sigma_{ij}^{\nu} = E^{ijkl\nu} \varepsilon_{kl}^{\nu}$$

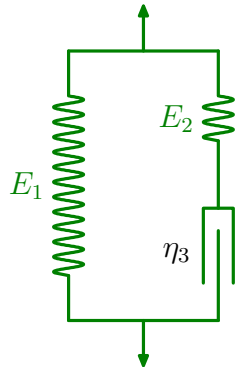


$$E^{ijkl\nu} = \begin{pmatrix} \Phi_{11} & 0 & 0 & 0 & \Phi_{12} & 0 & 0 & 0 & \Phi_{13} \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ \Phi_{21} & 0 & 0 & 0 & \Phi_{22} & 0 & 0 & 0 & \Phi_{23} \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ \Phi_{31} & 0 & 0 & 0 & \Phi_{32} & 0 & 0 & 0 & \Phi_{33} \end{pmatrix}_{\{ij\{kl\}}$$

$$\dot{\sigma}^{ab} + A^{ab}{}_{cd} \sigma^{cd} = B^{abcd} \dot{\varepsilon}^{cd} + D^{abcd} \varepsilon^{cd}$$

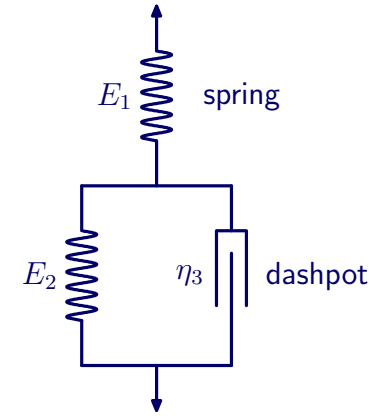
turn

Three element Zener model



$$\dot{\sigma} + \frac{E_2}{\eta_3} \sigma = (E_1 + E_2) \dot{\epsilon} + \frac{E_1 E_2}{\eta_3} \epsilon$$

Poynting-Thompson model



$$\dot{\sigma} + \frac{E_1 + E_2}{\eta_3} \sigma = E_1 \dot{\epsilon} + \frac{E_1 E_2}{\eta_3} \epsilon$$

$$\dot{\sigma} + a \sigma = b \dot{\epsilon} + d \epsilon$$

1D → 3D

$$\dot{\sigma}^{ab} + A^{ab}_{cd} \sigma^{cd} = B^{abcd} \dot{\epsilon}_{cd} + D^{abcd} \epsilon_{cd}$$

c^{ab} – a constant tensor

$$\sigma^{ab} = U^{ab}_{cd} \Lambda^{cd}_{ij} \left(\int \mathcal{L}^{ij}_{kl} \mathcal{U}^{kl}_{mn} (B^{mnop} \dot{\epsilon}_{op} + D^{mnop} \epsilon_{op}) dt + c^{ab} \right)$$

$$U^{ab}_{cd} = \left(\{M_1^{ab}\}_{ab}, \{M_2^{ab}\}_{ab}, \dots, \{M_9^{ab}\}_{ab} \right)_{ab[cd]}, U^{ab}_{cd} \mathcal{U}^{cd}_{kl} = \delta^a_k \delta^b_l$$

$$\lambda, M^{ab} - \text{eigenvalue problem: } (-A^{ab}_{cd} - \lambda \delta^a_c \delta^b_d) M^{cd} = 0$$

OCTAVE, MAXIMA: eigenvalue and eigenvector of the matrix $\{-A^{ab}_{cd}\}_{ab[cd]}$

$$\Lambda^{ab}_{cd} = \begin{pmatrix} e^{\lambda_1 t} & \dots & 0 \\ \vdots & \ddots & \vdots \\ 0 & \dots & e^{\lambda_9 t} \end{pmatrix}_{ab[cd]}, \quad \mathcal{L}^{ab}_{cd} = \begin{pmatrix} e^{-\lambda_1 t} & \dots & 0 \\ \vdots & \ddots & \vdots \\ 0 & \dots & e^{-\lambda_9 t} \end{pmatrix}_{ab[cd]}$$

isotropic

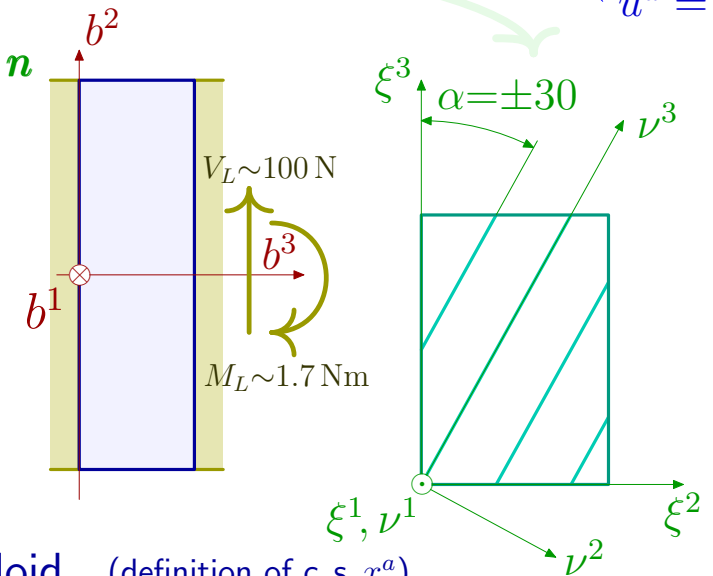
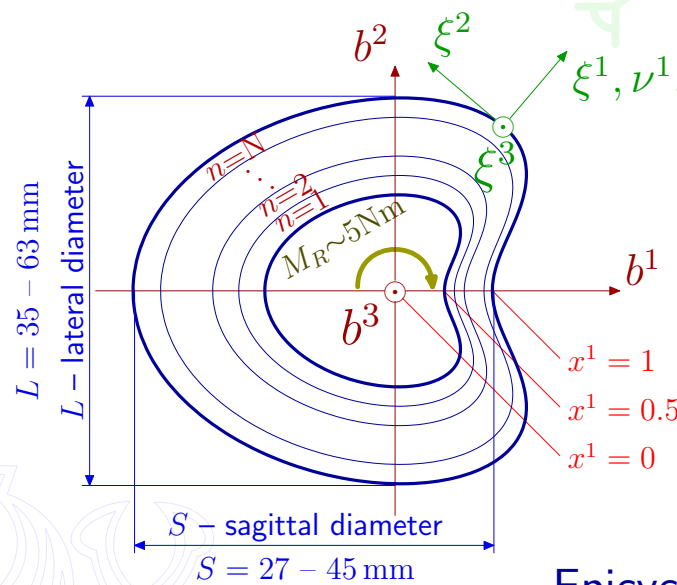
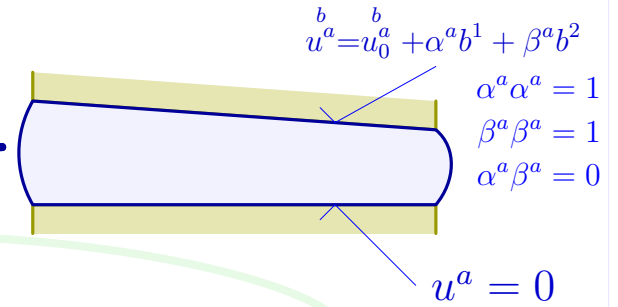
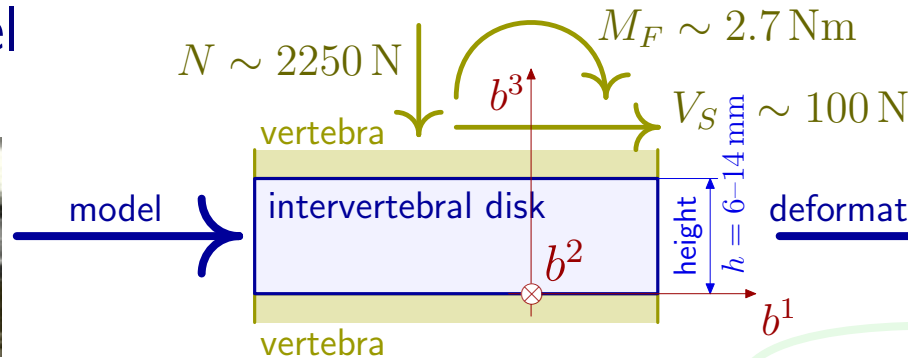
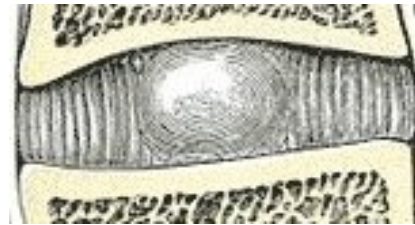
$$A^{ab}_{cd} = \alpha_1 g^{ab} g_{cd} + \alpha_2 (\delta^a_c \delta^b_d + \delta^a_d \delta^b_c)$$

$$B^{abcd} = \beta_1 g^{ab} g^{cd} + \beta_2 (g^{ac} g^{bd} + g^{ad} g^{bc})$$

$$D^{abcd} = \gamma_1 g^{ab} g^{cd} + \gamma_2 (g^{ac} g^{bd} + g^{ad} g^{bc})$$

Geometrical model

MARES & DANIEL (2006)



Epicycloid (definition of c. s. x^a)

$$\begin{aligned} b^1 &= 2r_x x^1 \cos x^2 - d_x x^1 \cos 2x^2 \\ b^2 &= 2r_y x^1 \sin x^2 - d_y x^1 \sin 2x^2 \\ b^3 &= x^3 \end{aligned}$$

$$\begin{aligned} 0 &\leq x^1 \leq 1 \\ 0 &\leq x^2 \leq 2\pi \\ 0 &\leq x^3 \leq h \end{aligned} \quad \begin{aligned} L, S + \text{shape} &\Rightarrow r_x, r_y, d_x, d_y \\ \text{number of lamellas: } &20 \\ \Delta x^1 &= 0.05 \end{aligned}$$

$$\frac{\partial b^a}{\partial x^b} = \begin{pmatrix} 2r_x \cos x^2 - d_x \cos 2x^2 & 2d_x x^1 \sin 2x^2 - 2r_x x^1 \sin x^2 & 0 \\ 2r_y \sin x^2 - d_y \sin 2x^2 & 2r_y x^1 \cos x^2 - 2d_y x^1 \cos 2x^2 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \mathbf{bx}$$

$$\frac{\partial x^a}{\partial b^b} = \left(\frac{\partial b^a}{\partial x^b} \right)^{-1} = \mathbf{xb}$$

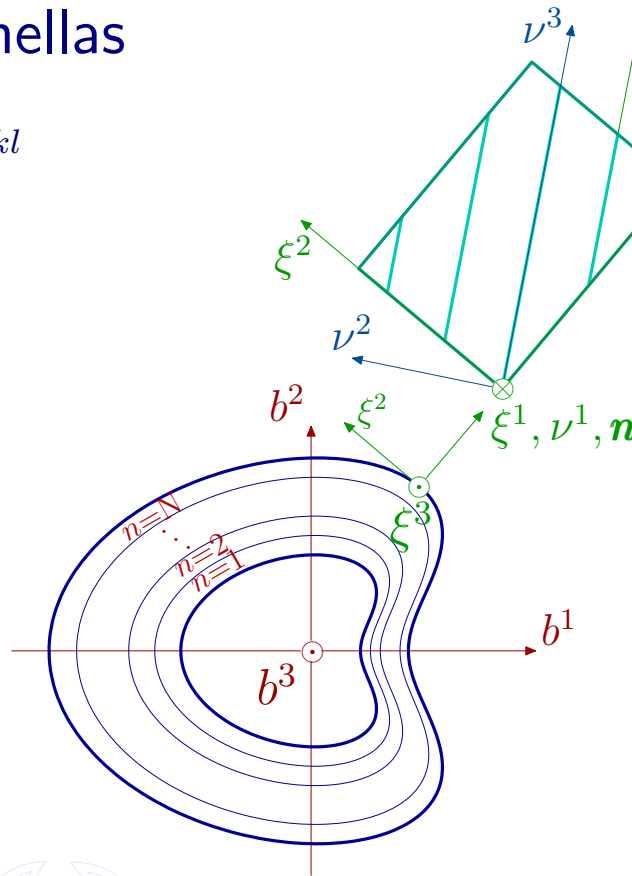
$$g_{ab}^x = \frac{\partial b^c}{\partial x^a} \frac{\partial b^d}{\partial x^b} g_{cd}^b = \mathbf{bx}'\mathbf{bx}$$

Elasticity tensor of the lamellas

$$E^{abcd} = \frac{\partial x^a}{\partial \nu^i} \frac{\partial x^b}{\partial \nu^j} \frac{\partial x^c}{\partial \nu^k} \frac{\partial x^d}{\partial \nu^l} E^{ijkl}$$

$$\frac{\partial x^a}{\partial \nu^i} = \frac{\partial x^a}{\partial b^b} \frac{\partial b^b}{\partial \xi^c} \frac{\partial \xi^c}{\partial \nu^i}$$

$$\frac{\partial x^a}{\partial b^b} = \left(\frac{\partial b^a}{\partial x^b} \right)^{-1}$$



$$\frac{\partial \xi^a}{\partial \nu^b} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \cos \alpha & \sin \alpha \\ 0 & -\sin \alpha & \cos \alpha \end{pmatrix}$$

Normal $\mathbf{n} = (n^1, n^2, 0)$

$$n^1 = \frac{b_t^2}{\sqrt{(b_t^1)^2 + (b_t^2)^2}}$$

$$n^2 = \frac{-b_t^1}{\sqrt{(b_t^1)^2 + (b_t^2)^2}} \quad \text{kde } t = \frac{\partial}{\partial x^2}$$

$$n^1 = \frac{N^1}{d}, \quad N^1 = 2r_y x^1 \cos x^2 - 2d_y x^1 \cos 2x^2$$

$$n^2 = \frac{N^2}{d}, \quad N^2 = 2r_x x^1 \sin x^2 - 2d_x x^1 \sin 2x^2$$

$$d = \sqrt{(N^1)^2 + (N^2)^2}$$

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```
xnu=xb*bxi*xinu
```

```
Ex=kron(xnu,xnu)*Enu*kron(xnu',xnu')
```

$$\frac{\partial \xi^a}{\partial b^b} = \begin{pmatrix} n^1 & n^2 & 0 \\ -n^2 & n^1 & 0 \\ 0 & 0 & 1 \end{pmatrix}, \quad \frac{\partial b^a}{\partial \xi^b} = \begin{pmatrix} n^1 & -n^2 & 0 \\ n^2 & n^1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

1. Small deformations, elastic nucleus pulposus

Isotropic elastic Nucleus pulposus (soft, incompressible)

$$E_{\text{polpus}}^{abcd} = \lambda g^{ab} g^{cd} + \mu g^{ac} g^{bd} + \mu g^{ad} g^{bc}$$

Minimum Π

2. Small deformations, viscoelastic nucleus pulposus

Isotropic viscoelastic Nucleus pulposus

$$\dot{\sigma}^{ab} + A^{ab}_{cd} \sigma^{cd} = B^{abcd} \dot{\varepsilon}_{cd} + D^{abcd} \varepsilon_{cd}$$

Galerkin method, base (BC)

$$\nabla_a \sigma^{ab} = 0, 2\varepsilon_{ab} = \nabla_a u_b + \nabla_b u_a$$

$$f(\sigma^{ab}, \dot{\sigma}^{ab}, \varepsilon^{ab}, \dot{\varepsilon}^{ab}) = 0$$

3. Large deformations, elastic nucleus pulposus

$$2E_{ab} = \nabla_a u_b + \nabla_b u_a + \nabla_a u^c \nabla_b u_c$$

$$E_{\text{polpus}}^{abcd} = \lambda g^{ab} g^{cd} + \mu g^{ac} g^{bd} + \mu g^{ad} g^{bc}$$

Minimum Π

4. Large deformations, viscoelastic nucleus pulposus

$$2E_{ab} = \nabla_a u_b + \nabla_b u_a + \nabla_a u^c \nabla_b u_c$$

$$\text{Nucleus: } \dot{\Sigma}^{ab} + A^{ab}_{cd} \Sigma^{cd} = B^{abcd} \dot{E}_{cd} + D^{abcd} E_{cd}$$

Galerkin method, base (BC)

$$\nabla_a \Sigma^{ab} = 0$$

1. Small deformations, elastic nucleus pulposus

SYNGE, J. L. and SCHILD, A. (1978)

LOVELOCK, D. and RUND, H. (1989)

CIARLET, P. G. (2005)

GNU MAXIMA *.mac

Principle of the total potential energy minimum

$$\hat{u}_a = \arg \min_{u_b \in \Omega} \Pi(u_c)$$

$$\varepsilon_{ab} = \frac{1}{2} (\nabla_a u_b + \nabla_b u_a)$$

The real state of a deformed body minimizes the total potential energy (on a set of admissible states,)

$$\Pi(u_a) = a(u_a) - l(u_a)$$

$$\nabla_a u_b = \partial_a u_b - \Gamma_{ab}^c u_c$$

Christoffel symbol of the 2nd kind

The elastic strain energy

$$\Gamma_{ab}^d = g^{dc} \frac{1}{2} (g_{ac,b} + g_{cb,a} - g_{ab,c})$$

$$a(u_a) = \frac{1}{2} \int_{\Omega} E^{abcd} \varepsilon_{ab}(u_a) \varepsilon_{cd}(u_a) d\Omega$$

$$\varepsilon_{ab}^x = \frac{1}{2} (\partial_a^x u_b + \partial_b^x u_a - 2 \Gamma_{ab}^c u_c^x)$$

The potential energy of the applied forces $p^a(\frac{N}{mm^3}), t^a(\frac{N}{mm^2})$

$$l(u_a) = \int_{\Omega} p^a u_a d\Omega + \int_{\partial_t \Omega} t^a u_a d\Gamma$$

$$\Gamma_{ab}^c u_c^x = \Gamma_{ab}^1 u_1^x + \Gamma_{ab}^2 u_2^x + \Gamma_{ab}^3 u_3^x$$

In the coordinate system x :

$$\Gamma_{22}^1 = - \frac{(2d_x r_y + 4d_y r_x) \sin x^2 \sin 2x^2 + (4d_x r_y + 2d_y r_x) \cos x^2 \cos 2x^2 - 2r_x r_y - 4d_x d_y}{(2d_x r_y + d_y r_x) \sin x^2 \sin 2x^2 + (d_x r_y + 2d_y r_x) \cos x^2 \cos 2x^2 - 2r_x r_y - d_x d_y} x^1$$

$$\Gamma_{12}^2 = \Gamma_{21}^2 = \frac{1}{x^1}$$

$$\Gamma_{22}^2 = - \frac{3d_y r_x \cos x^2 \sin 2x^2 - 3d_x r_y \sin x^2 \cos 2x^2}{(2d_x r_y + d_y r_x) \sin x^2 \sin 2x^2 + (d_x r_y + 2d_y r_x) \cos x^2 \cos 2x^2 - 2r_x r_y - d_x d_y}$$

$$\Gamma_{ab}^1 = \begin{pmatrix} 0 & 0 & 0 \\ 0 & \Gamma_{22}^1 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

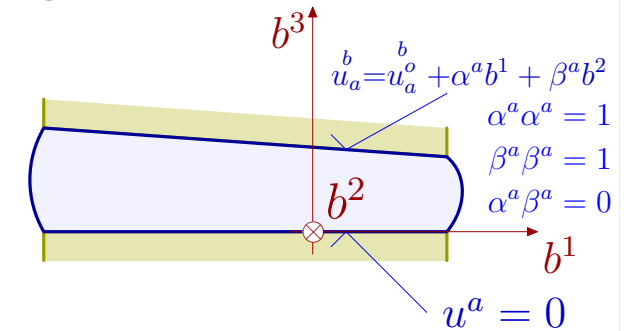
$$\Gamma_{ab}^2 = \begin{pmatrix} 0 & \Gamma_{12}^2 & 0 \\ \Gamma_{21}^2 & \Gamma_{22}^2 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\Gamma_{ab}^3 = 0$$

Fourier series expansion

$$u_a^x = \sum_{k,l=-\infty}^{\infty} \sum_{m=1}^{\infty} U_a^{klm} \left(e^{i2\pi kx^1} e^{ilx^2} x^1 + 1 \right) \sin \frac{\pi mx^3}{h} + u_a^h \frac{x^3}{h}$$

BC



where

$$u_a^h = \frac{\partial b^b}{\partial x^a} u_b^h$$

Small deformation: $\varphi_1, \varphi_2, \varphi_3 \rightarrow 0$

The upper vertebra moves as a rigid body

$$u_b^h: \begin{aligned} u_1^h &= u_1^o - b^2 \varphi_3 \\ u_2^h &= u_2^o + b^1 \varphi_3 \\ u_3^h &= u_3^o - b^1 \varphi_2 + b^2 \varphi_1 \end{aligned}$$

φ_1 – rotation around axis b^1

φ_2 – rotation around axis b^2

φ_3 – rotation around axis b^3

u_a^o – displacement of the point $b^{1,2} = 0, b^3 = h$

$$u_a^x = \mathbf{u} \mathbf{x} = \mathbf{N} * \mathbf{U}$$

\mathbf{U} – unknown coefficients of the F. series

GNU OCTAVE

`B=B(x1, x2, x3)`

`B=kron(kron(e.^ (i*k*2*pi*x1), x1*e.^ (i*1*x2)+1), sin(pi*m*x3/h))`

`bc=[0,0,-b(2); 0,0,b(1); b(2),-b(1),0];`

`N=[[B,zeros(1,K),zeros(1,K); zeros(1,K),B,zeros(1,K);`

`zeros(1,K),zeros(1,K),B], bx*x3/h, bx*bc*x3/h]; ## size(U)=3*K+6`

$$\mathbf{U} = \begin{pmatrix} \bar{u} \\ b \\ u_1^o \\ b \\ u_2^o \\ b \\ u_3^o \\ b \\ \varphi_1 \\ \varphi_2 \\ \varphi_3 \end{pmatrix}$$

Elastic energy

CIARLET, P. G. (2005)

LOVELOCK, D. and RUND, H. (1989)

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GNU MAXIMA *.mac

$$a(u_a) = \frac{1}{2} \int_{\Omega} E^{abcd} \varepsilon_{ab}(u_a) \varepsilon_{cd}(u_a) d\Omega$$

$$\varepsilon_{ab} = \frac{1}{2} (\nabla_a u_b + \nabla_b u_a)$$

$$\nabla_a u_b = \partial_a u_b - \Gamma_{ab}^c u_c$$

$$E^{abcd} = E^{bacd} \Rightarrow \varepsilon_{ab}^x \rightarrow \partial_a^x u_b - \Gamma_{ab}^c u_c$$

$$a = \frac{1}{2} \int_{\Omega} \left(\partial_a^x u_b - \Gamma_{ab}^p u_p \right) E^{abcd} \left(\partial_c^x u_d - \Gamma_{cd}^p u_p \right) |g_{ab}^x|^{\frac{1}{2}} d^3x$$

$$\left\{ \partial_a^x u_b \right\}_{ab\uparrow} = DG * U$$

DG... GNU OCTAVE *.m

$$\left\{ \Gamma_{ab}^p u_p \right\}_{ab\uparrow} = Gamma * U$$

Gamma = vec(G1') * N(1, :) + vec(G2') * N(2, :)

$$G1 = \{ \Gamma_{ab}^1 \}_{a\uparrow b}$$

$$G2 = \{ \Gamma_{ab}^2 \}_{a\uparrow b}$$

$$a = \frac{1}{2} U' * K * U$$

numerical integration K

$$a = \frac{1}{2} U' * \int_0^1 \int_0^{2\pi} \int_0^h (DG(x)' - Gamma(x)') * Ex(x) * (DG(x) - Gamma(x)) * sqrt(det(gx(x))) dx^1 dx^2 dx^3 * U$$

Right hand side

$$l(u_a) = \int_{\Omega} p^a u_a d\Omega + \int_{\partial_t \Omega} t^a u_a d\Gamma$$

$$l = F * U$$

$$F = [\text{zeros}(3 * K, 1); V_s; V_l; -N; M_l; M_f; M_r]$$

$$U = \begin{pmatrix} \bar{U} \\ b \\ u_1^0 \\ b \\ u_2^0 \\ b \\ u_3^0 \\ \varphi_1 \\ \varphi_2 \\ \varphi_3 \end{pmatrix}$$

Necessary condition of minimum

$$\frac{\partial \Pi}{\partial U} = 0$$

$$\Pi = \frac{1}{2} U' * K * U - F * U$$

$$K * U = F$$

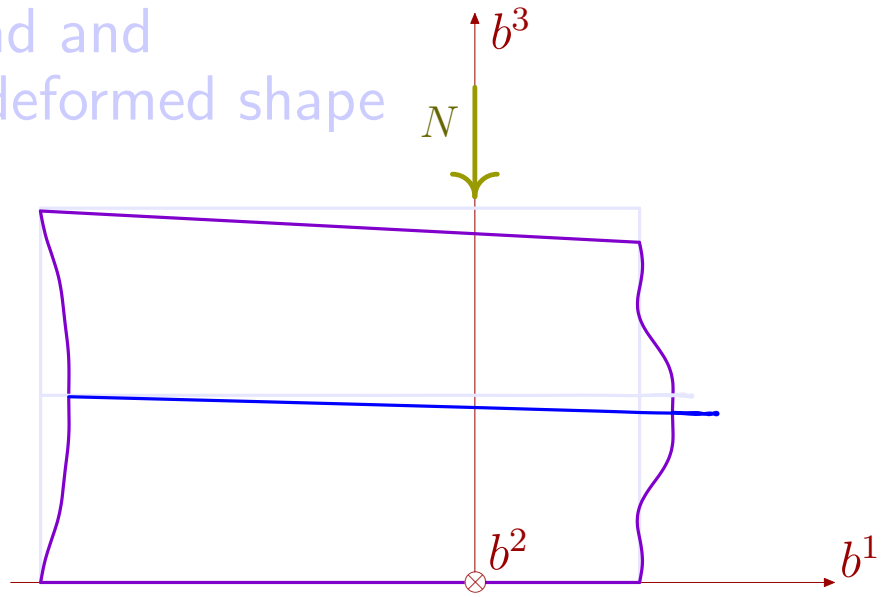
$$U = K^{-1} * F$$

$$u_a^x = \text{ux} = \text{real}(N(x) * U)$$

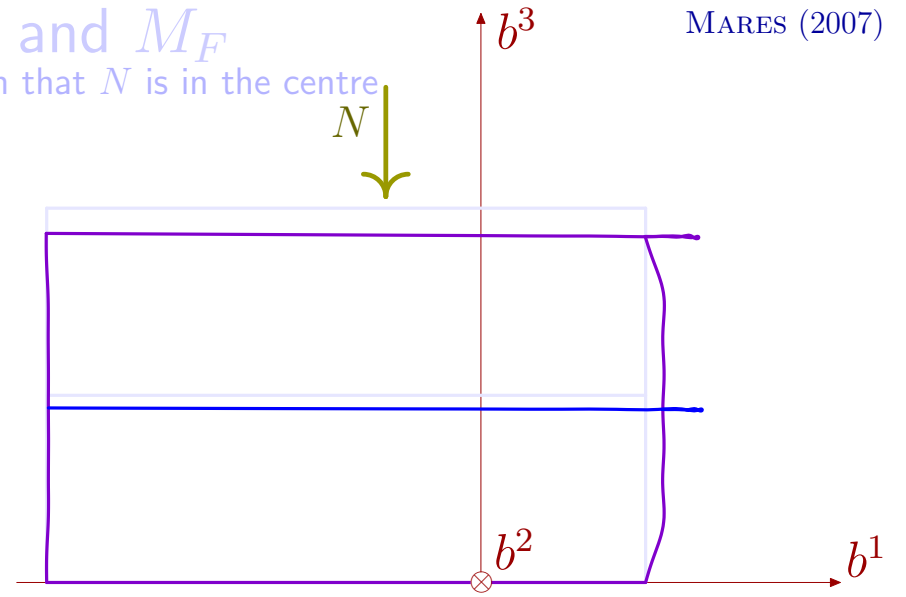
$$u_a^b = \frac{\partial x^b}{\partial b^a} u_b^x$$

$$u_b = x_b' * u_x$$

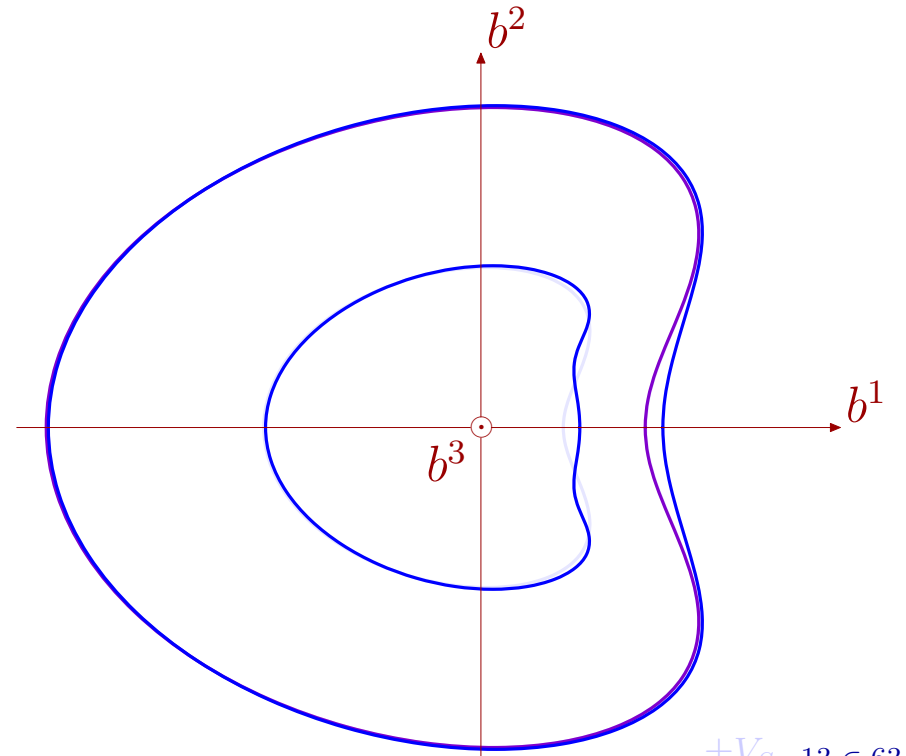
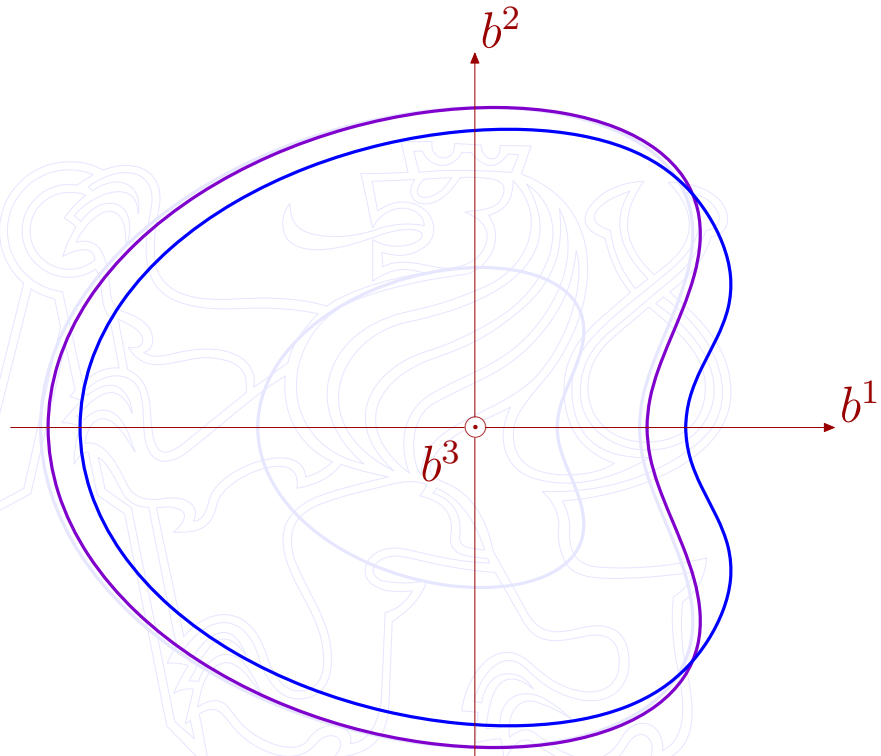
Load and deformed shape



N and M_F
such that N is in the centre



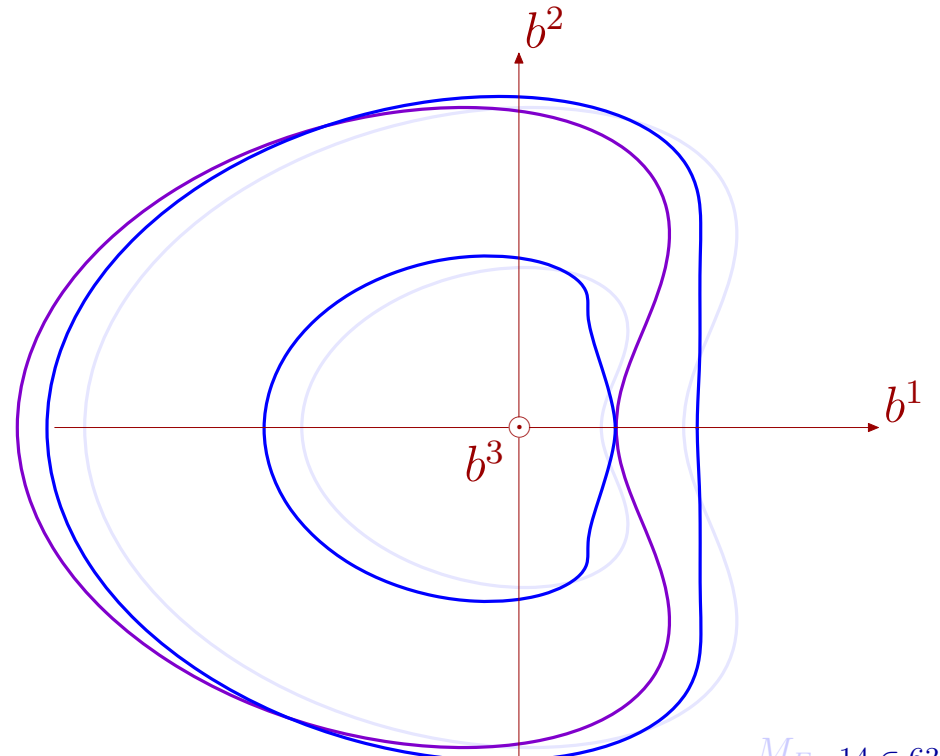
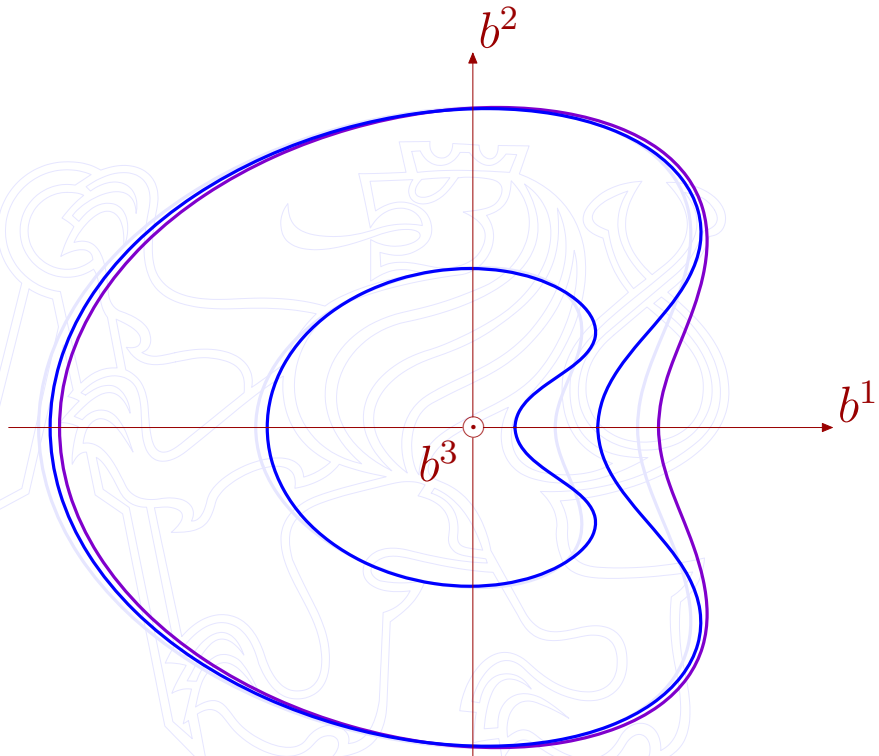
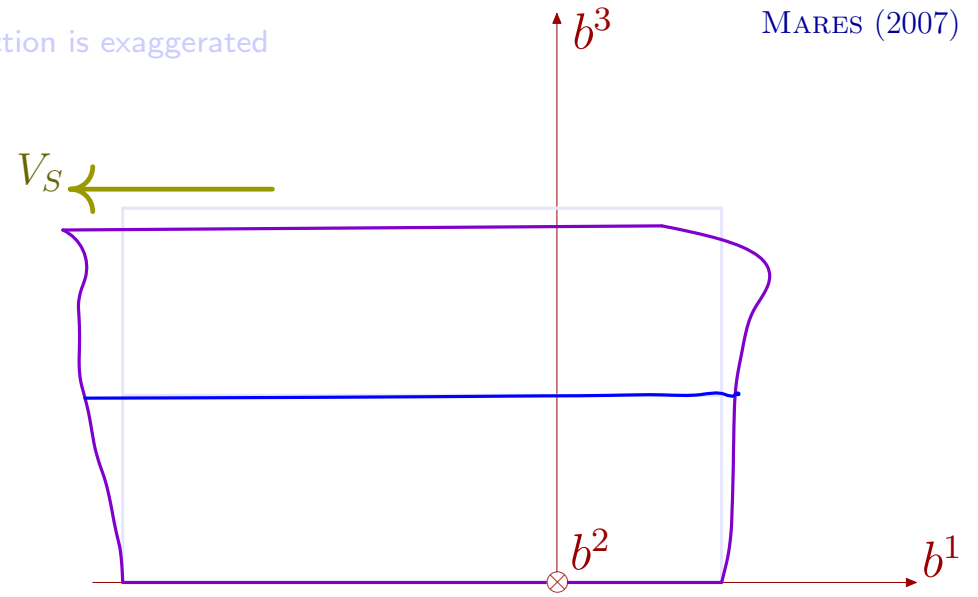
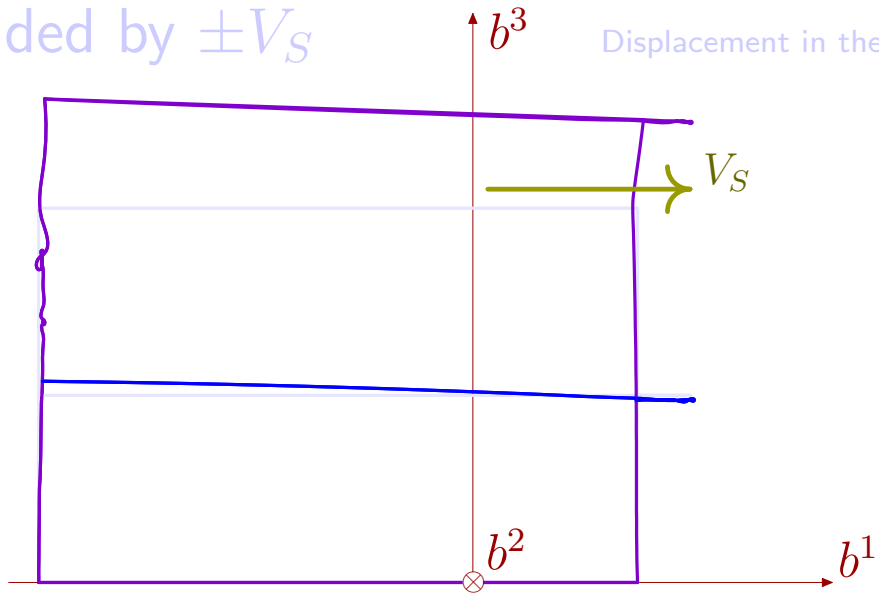
MARES (2007)



Loaded by $\pm V_S$

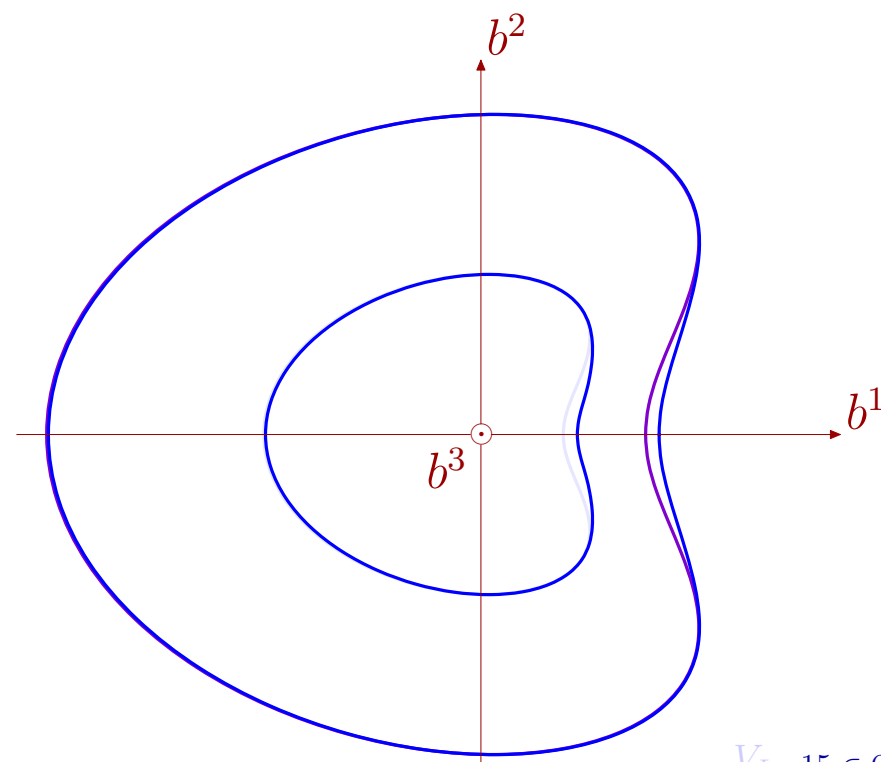
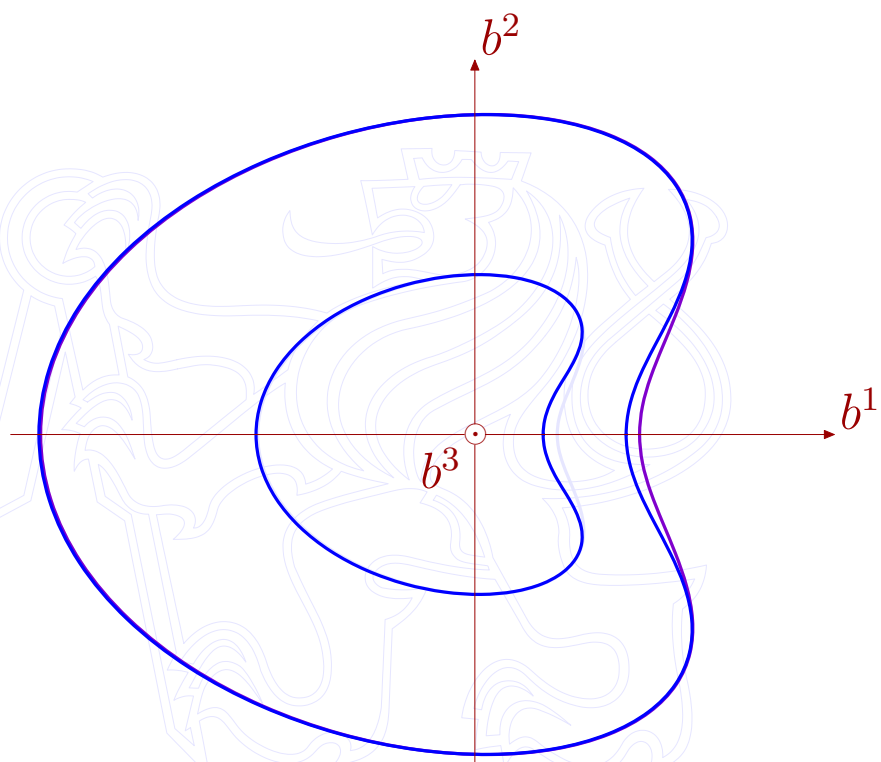
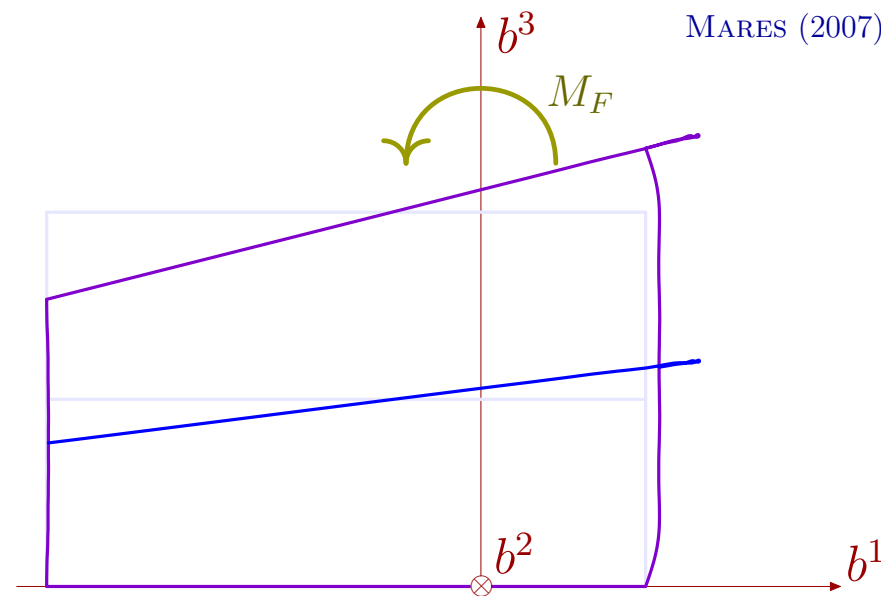
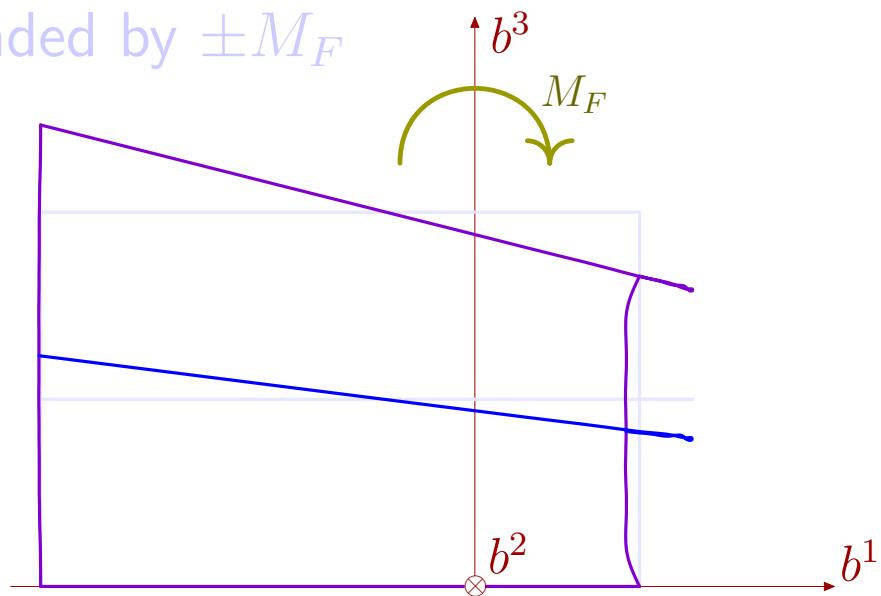
Displacement in the b^3 direction is exaggerated

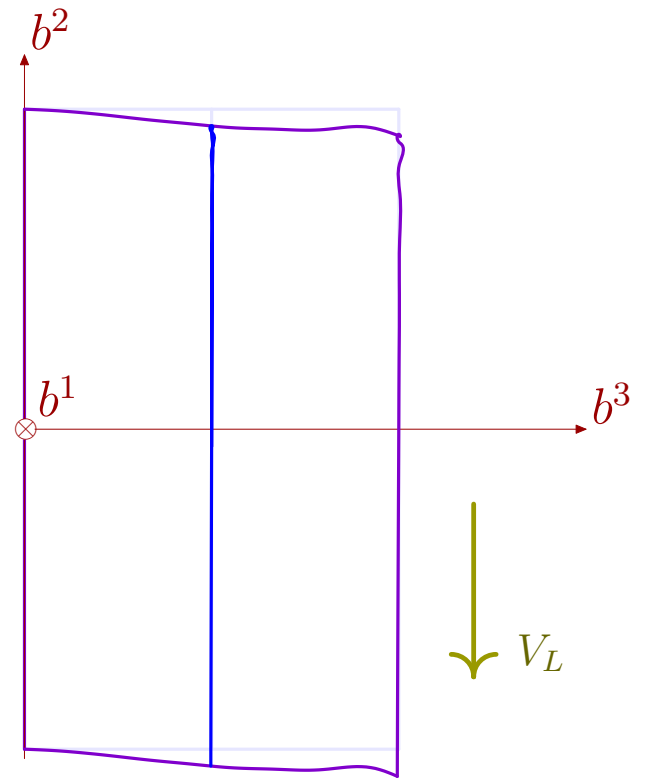
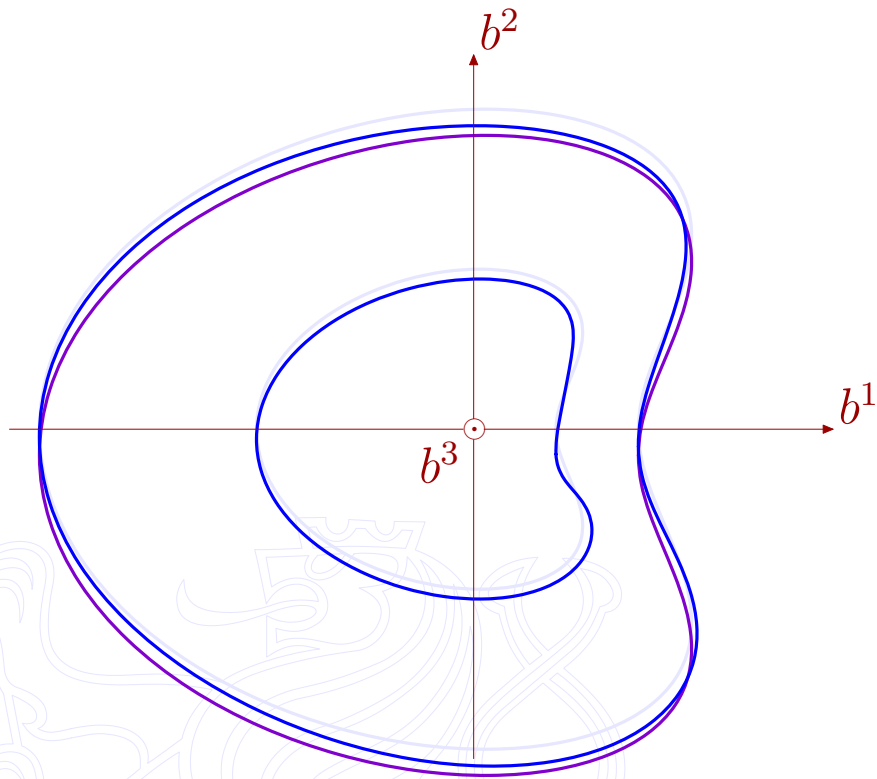
MARES (2007)

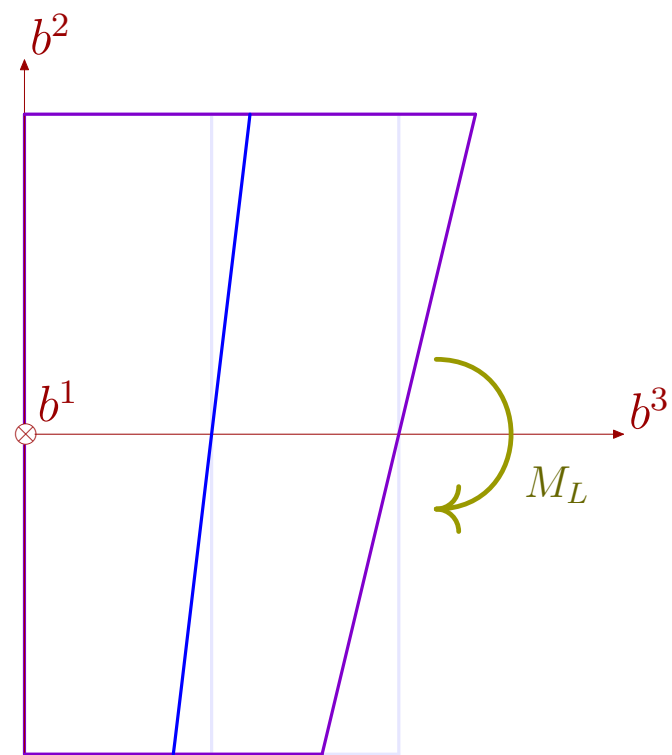
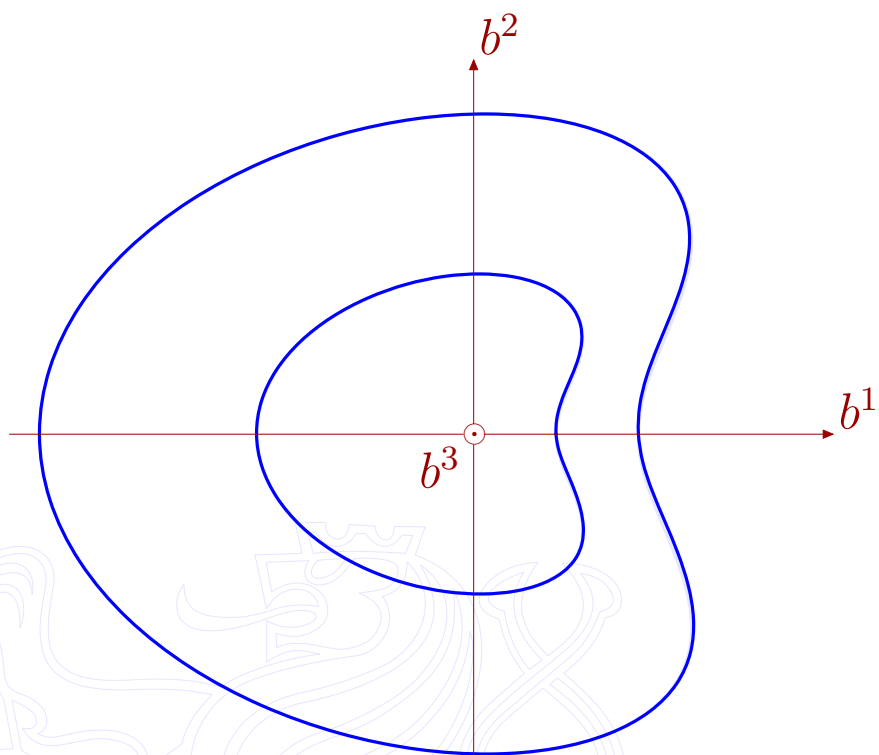


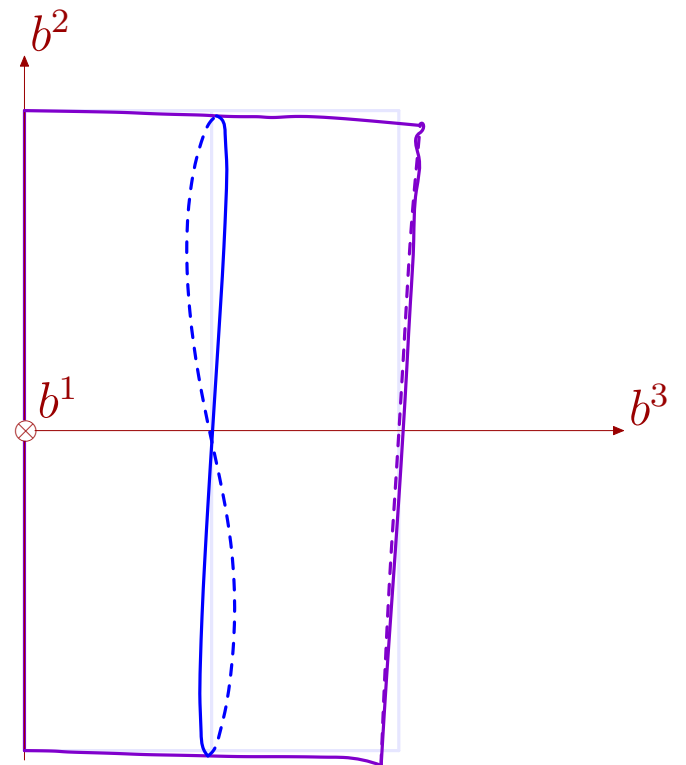
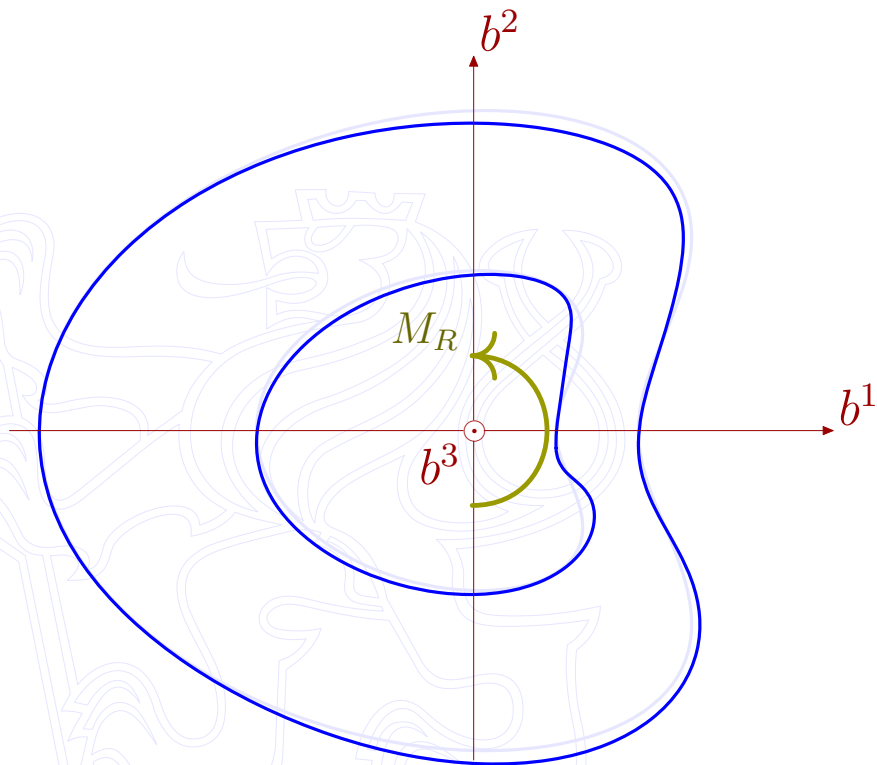
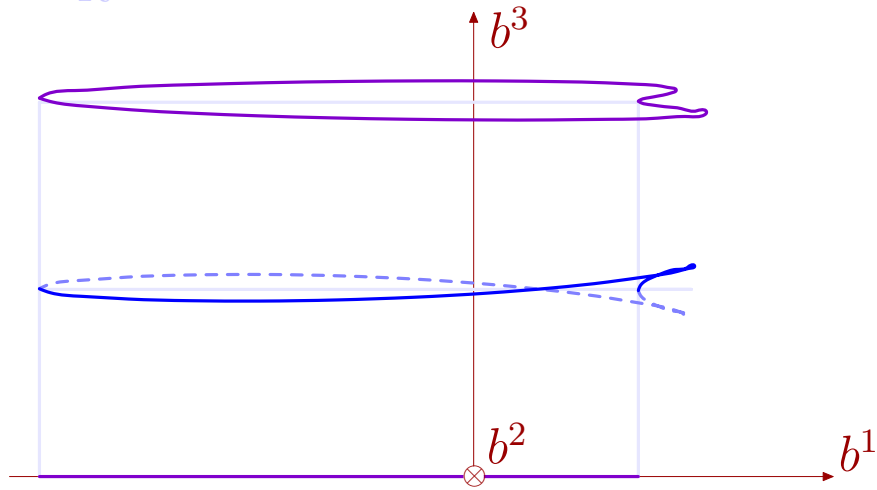
Loaded by $\pm M_F$

MARES (2007)

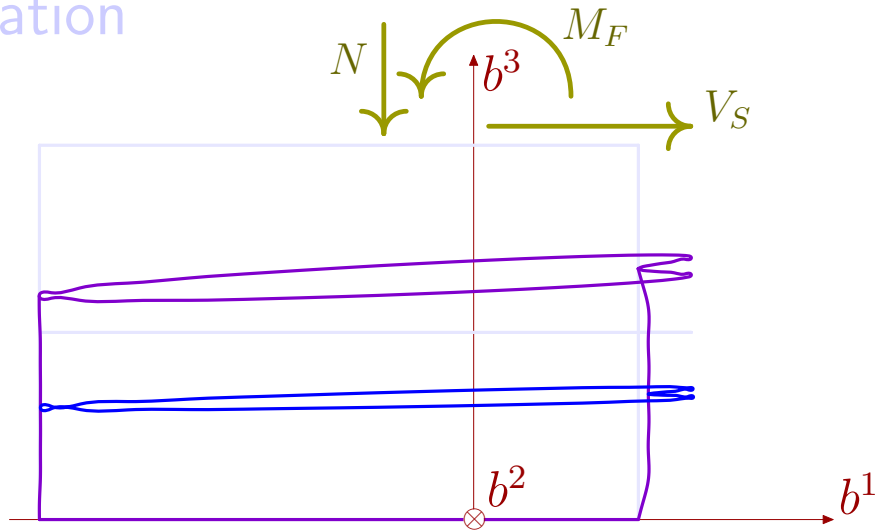








Deformation



full set of the loads

MARES (2007)

$$N = 2250 \text{ N}$$

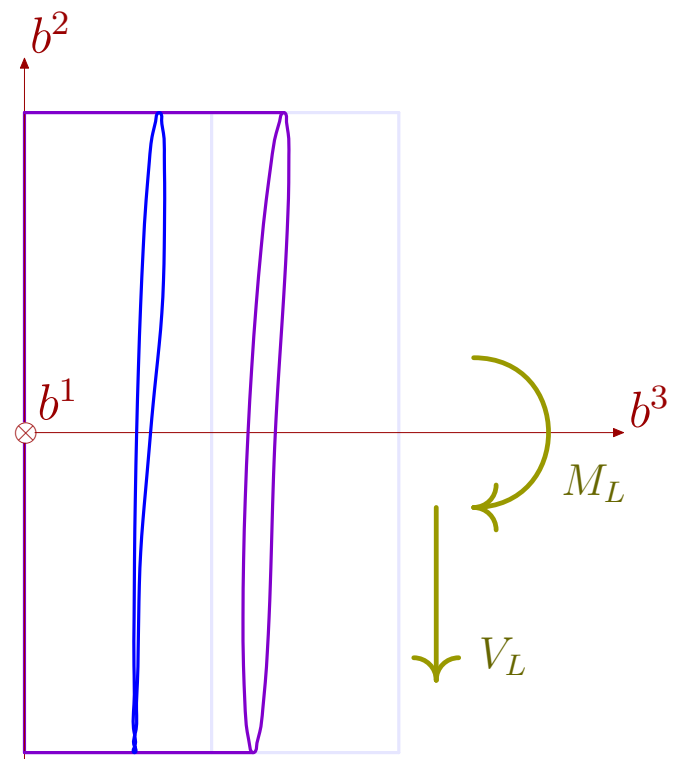
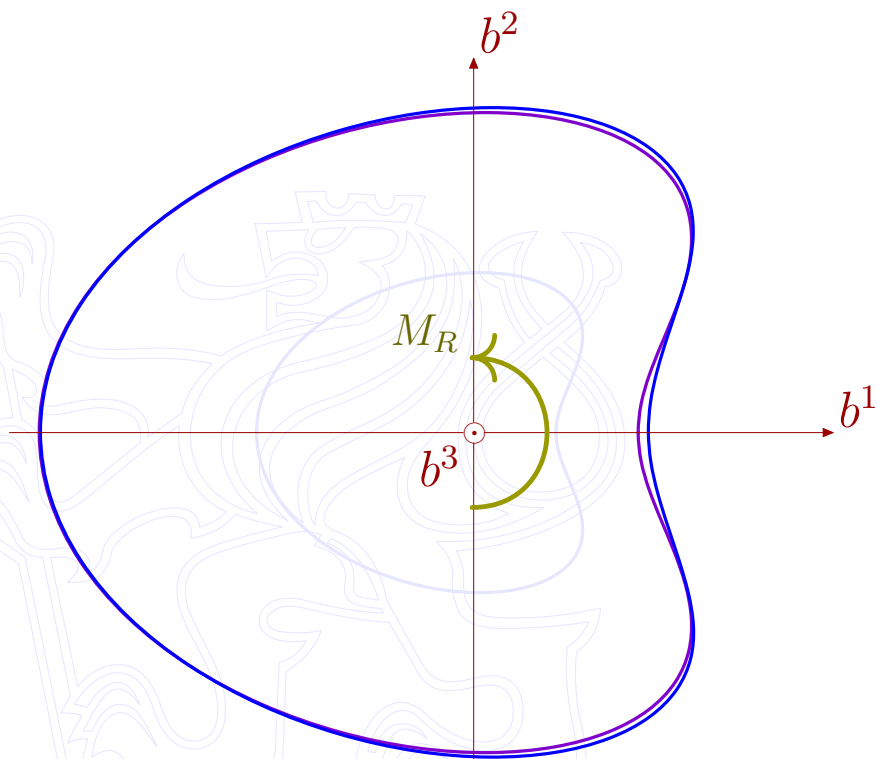
$$M_F = 2700 \text{ Nm}$$

$$V_S = 100 \text{ N}$$

$$M_R = 0 \text{ Nm}$$

$$V_L = 100 \text{ N}$$

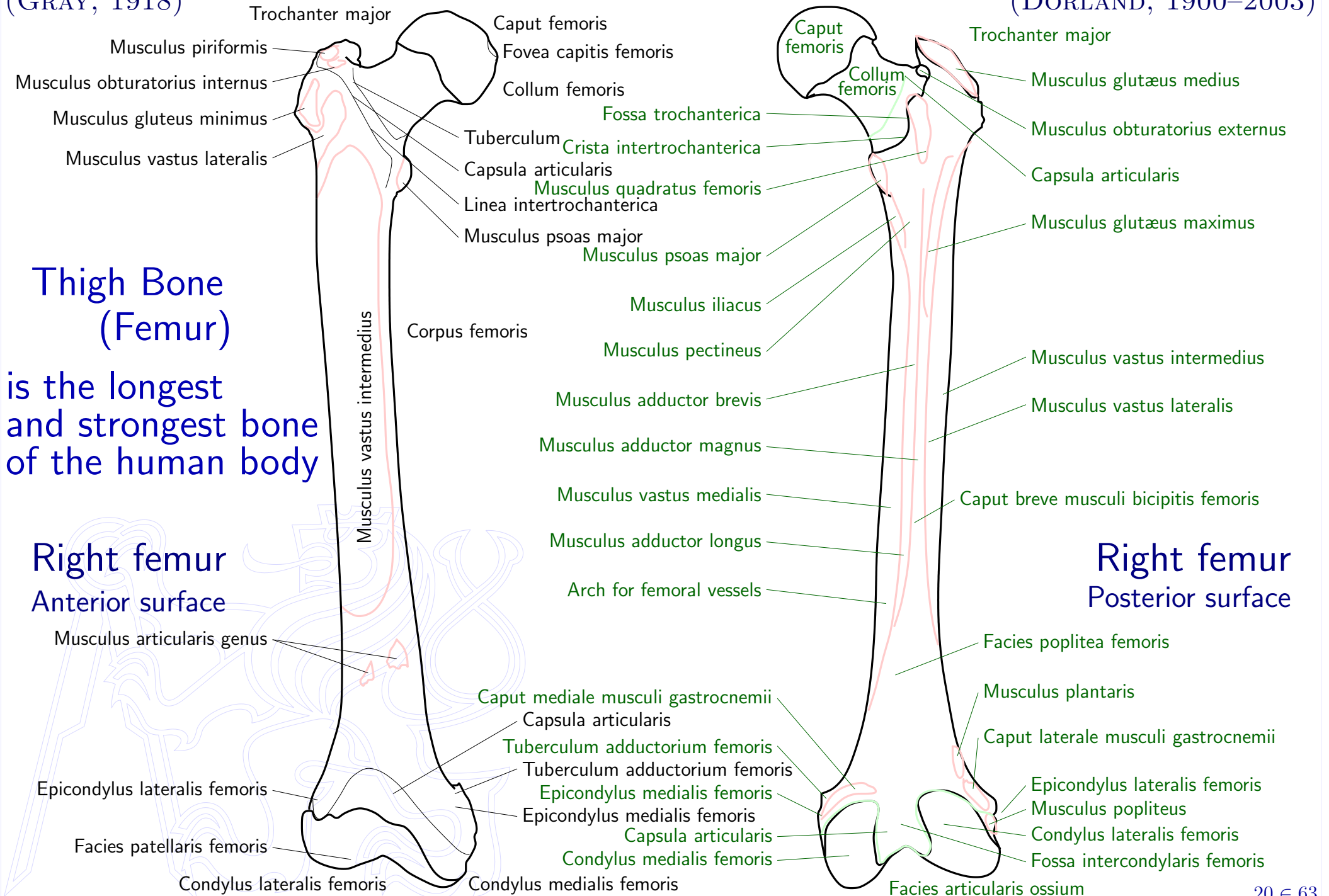
$$M_L = 1700 \text{ Nm}$$



Cortical bone (compact bone), the shaft of a long bone, as a fibre composite

(GRAY, 1918)

(DORLAND, 1900–2003)



Thigh Bone (Femur)
is the longest and strongest bone of the human body

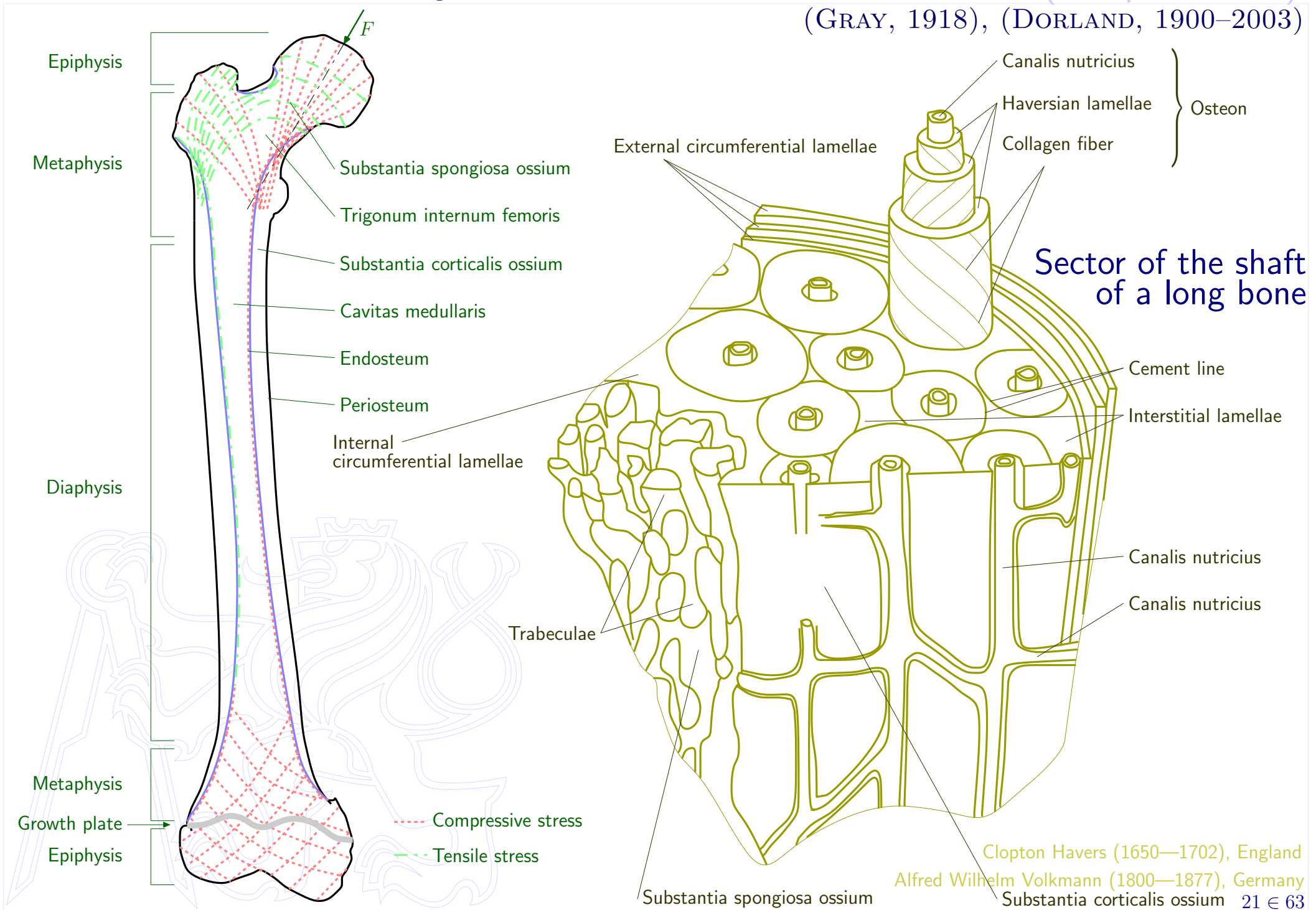
Right femur
Anterior surface

Right femur
Posterior surface

Internal structure of the right femur

Cortical bone (compact bone)

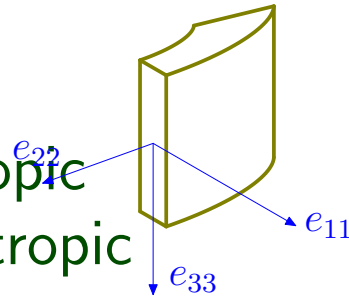
(GRAY, 1918), (DORLAND, 1900–2003)



There are several models of the cortical bone amongst them the

Model of cortical bone

- ⊙ homogeneous isotropic model
- ⊙ homogeneous transversely isotropic
- ⊙ homogeneous cylindrically orthotropic (axis, r , t)



$$E = \begin{pmatrix} e_{11} & e_{12} & e_{13} & 0 & 0 & 0 \\ e_{12} & e_{22} & e_{23} & 0 & 0 & 0 \\ e_{13} & e_{23} & e_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & e_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & e_{55} & 0 \\ 0 & 0 & 0 & 0 & 0 & e_{66} \end{pmatrix}$$

- [1] – (GOLDMANN, 2006)
- [2] – (ORÍAS, 2005)
- [3] – (YOON, KATZ, 1976)
- [4] – (KATZ *et al.*, 1984)
- [5] – (ASHMAN *et al.*, 1984)
- [6] – (RHO, 1996)
- [7] – (TAYLOR *et al.*, 2002)
- [8] – (BUSKIRK *et al.*, 1981)
- [9] – (MAHARIDGE, 1984)
- [0] – (LANG, 1970)

$$\left\{ \begin{matrix} z \\ \varepsilon_{ij}^{kl} \end{matrix} \right\}_{\{ij[kl]\}} = \begin{pmatrix} e_{11} & 0 & 0 & 0 & e_{12} & 0 & 0 & 0 & e_{13} \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ e_{12} & 0 & 0 & 0 & e_{22} & 0 & 0 & 0 & e_{23} \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ e_{13} & 0 & 0 & 0 & e_{23} & 0 & 0 & 0 & e_{33} \end{pmatrix}$$

The average material characteristics of these models are determined experimentally (mechanical, using acoustic waves)

(ORÍAS, 2005), (GOLDMANN, 2006)

The entries of the stiffness tensor [GPa] (dry/fresh bovine/human)

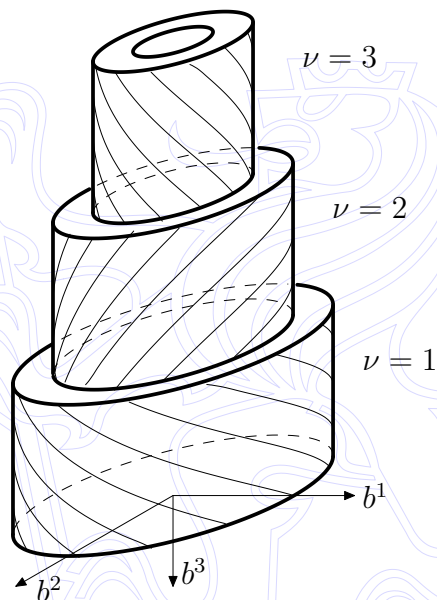
[GPa]	[1]	[2]	[3]	[4]	[5]	[6]	[7]	[8]	[9]	[0]
e_{11}	27.4±1.6	16.75±2.27	23.4±0.0031	21.2±0.5	18.0	19.4±1.3	24.89	14.1	22.4	19.7
e_{22}	30.3±2.8	19.66±2.09	24.1±0.0035	21.0±1.4	20.2	20.0±1.4	26.16	18.4	25.0	19.7
e_{33}	34.1±1.7	27.33±1.64	32.5±0.0044	29.0±1.0	27.6	30.9±1.9	33.20	25.0	35.0	32.0
e_{44}	9.3± 0.9	6.22±0.31	8.7±0.0013	6.3±0.4	6.23	5.7±0.5	7.11	7.0	8.2	5.4
e_{55}	7.0± 0.4	5.65±0.53	6.9±0.0012	6.3±0.2	5.6	5.2±0.6	6.58	6.3	7.1	5.4
e_{66}	6.9± 0.5	4.64±0.43	7.2±0.0011	5.4±0.2	4.5	4.1±0.5	5.71	5.28	6.1	3.8
e_{12}	9.1		9.1±0.0038	11.7±0.7	10.0	11.3±0.1	11.18	6.34	14.0	12.1
e_{13}	8.3±5.3		9.1±0.0055	11.1±0.8	10.1	12.5±0.1	13.59	4.84	15.8	12.6
e_{23}	8.5		9.2±0.0055	12.7±0.8	10.7	12.6±0.1	13.84	6.94	13.6	12.6

Let us build up a methodology of another model of cortical bone, say **Heterogeneous locally orthotropic model of cortical bone** (because of the locality of the orthotropy the model is in essence anisotropic)

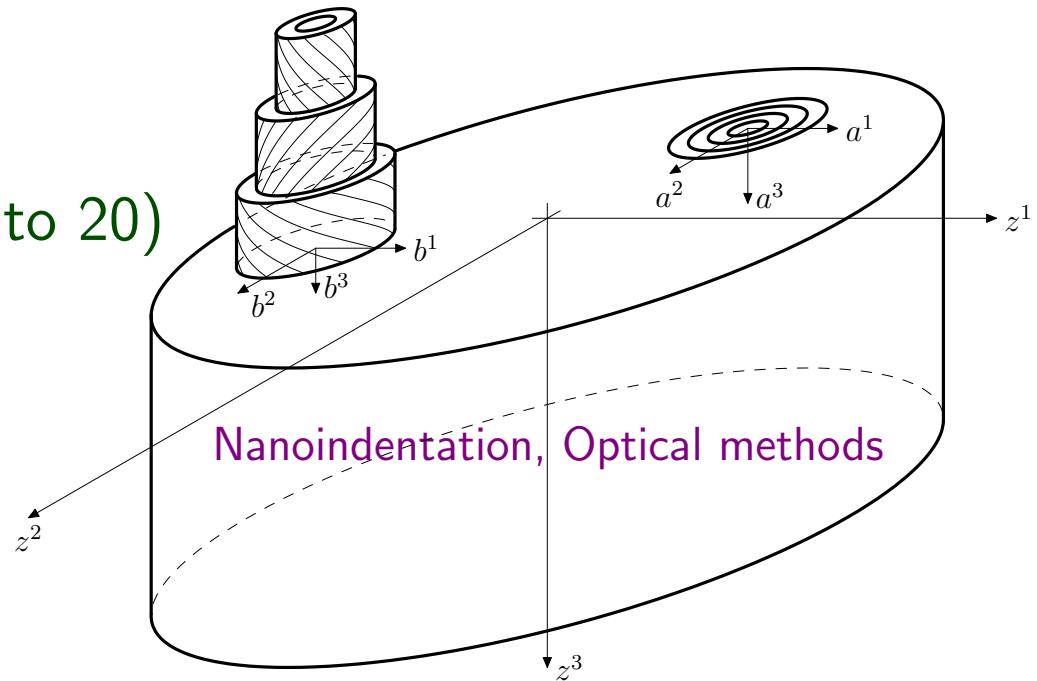
The basic unit of compact bone is known as the osteon

The osteon consist of a number (4 to 20) **Haversian lamellae**

Each of these lamellae is 3 to 7 microns thick
Clopton Havers (1650—1702), England



Osteon, the global coordinate systyem



Cortical bone and two of the osteons

The lamellae are composed of collagen fibers that are winded under an angle α changing from one lamella to another lamella

Elasticity tensor E^{abcd} in the c.s. of the local orthotropy

CIARLET, P. G. (2005)

MAREŠ, T. (2006)

in Cartesian coordinate system ν^a

$$\sigma^{ij} = E^{ijkl} \varepsilon_{kl}$$

$$\left\{ E^{ijkl} \right\}_{\{ij\{kl\}} = \begin{pmatrix} \Phi_{1111} & 0 & 0 & 0 & \Phi_{1122} & 0 & 0 & 0 & \Phi_{1133} \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & G_{12} & 0 & G_{12} & 0 & 0 & 0 & 0 & 0 \\ \Phi_{2211} & 0 & 0 & 0 & \Phi_{2222} & 0 & 0 & 0 & \Phi_{2233} \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ 0 & 0 & G_{13} & 0 & 0 & 0 & G_{13} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & G_{23} & 0 & G_{23} & 0 \\ \Phi_{3311} & 0 & 0 & 0 & \Phi_{3322} & 0 & 0 & 0 & \Phi_{3333} \end{pmatrix}$$

$$E^{abcd} = E^{bacd}$$

$$\Phi_{1111} = \frac{1 - \nu_{23}\nu_{32}}{N} E_{11}, \quad \Phi_{1122} = \frac{\nu_{21} + \nu_{23}\nu_{31}}{N} E_{11}, \quad \Phi_{1133} = \frac{\nu_{31} + \nu_{32}\nu_{21}}{N} E_{11}$$

$$\Phi_{2211} = \frac{\nu_{12} + \nu_{13}\nu_{32}}{N} E_{22}, \quad \Phi_{2222} = \frac{1 - \nu_{13}\nu_{31}}{N} E_{22}, \quad \Phi_{2233} = \frac{\nu_{32} + \nu_{31}\nu_{12}}{N} E_{22}$$

$$\Phi_{3311} = \frac{\nu_{13} + \nu_{12}\nu_{23}}{N} E_{33}, \quad \Phi_{3322} = \frac{\nu_{23} + \nu_{21}\nu_{13}}{N} E_{33}, \quad \Phi_{3333} = \frac{1 - \nu_{12}\nu_{21}}{N} E_{33}$$

$$N = 1 - \nu_{12}\nu_{21} - \nu_{23}\nu_{32} - \nu_{31}\nu_{13} - \nu_{12}\nu_{23}\nu_{31} - \nu_{13}\nu_{32}\nu_{21}$$

$$\text{Energy } (E^{abcd} = E^{cdab}) \Rightarrow \Phi_{1122} = \Phi_{2211} \Rightarrow \nu_{21} E_{11} = \nu_{12} E_{22}, \text{ etc.}$$

To build up the potential energy we will use the concept of locally orthotropic material
 Everything follows from the used coordinate systems

1. Local coordinate system of the orthotropy (ν^a)
2. Global coordinate system (z^a) that is common for the whole model
3. A sequence of working coordinates

The global transformation rule is given by the sequence of transformation rules

$$E^{abcd} = \frac{\partial z^a}{\partial \nu^i} \frac{\partial z^b}{\partial \nu^j} E^{ijkl} \frac{\partial z^c}{\partial \nu^k} \frac{\partial z^d}{\partial \nu^l}$$

The principle of minimum total potential energy

$$\hat{\mathbf{u}} = \arg \min_{\mathbf{u} \in \mathcal{K}} \Pi(\mathbf{u}), \text{ kde}$$

$$\Pi(\mathbf{u}) = a(\mathbf{u}, \mathbf{u}) - l(\mathbf{u})$$

$$a = \int_{\Omega} \varepsilon_{ab} \varepsilon_{cd} E^{abcd} d\Omega, \quad l(\mathbf{u}) = \int_{\Omega} p^i u_i d\Omega + \int_{\partial_t \Omega} t^i u_i d\Gamma$$

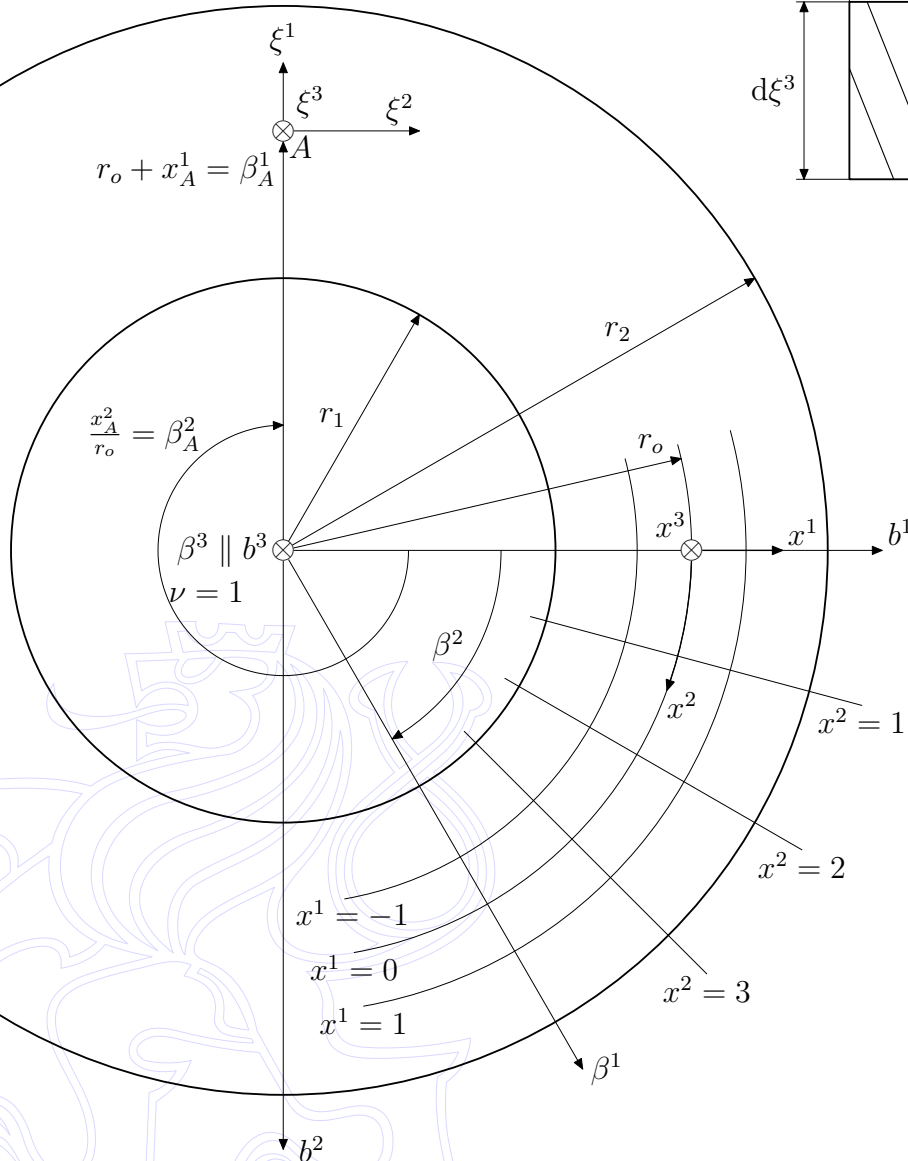
$$d\Omega = \left| g_{\alpha\beta} \right|^{\frac{1}{2}} d^3\beta, \quad d\Gamma = \left| h_{\alpha\beta} \right|^{\frac{1}{2}} d^2\phi$$

The concept of local orthotropy is very suitable for the detailed description of the bone behaviour

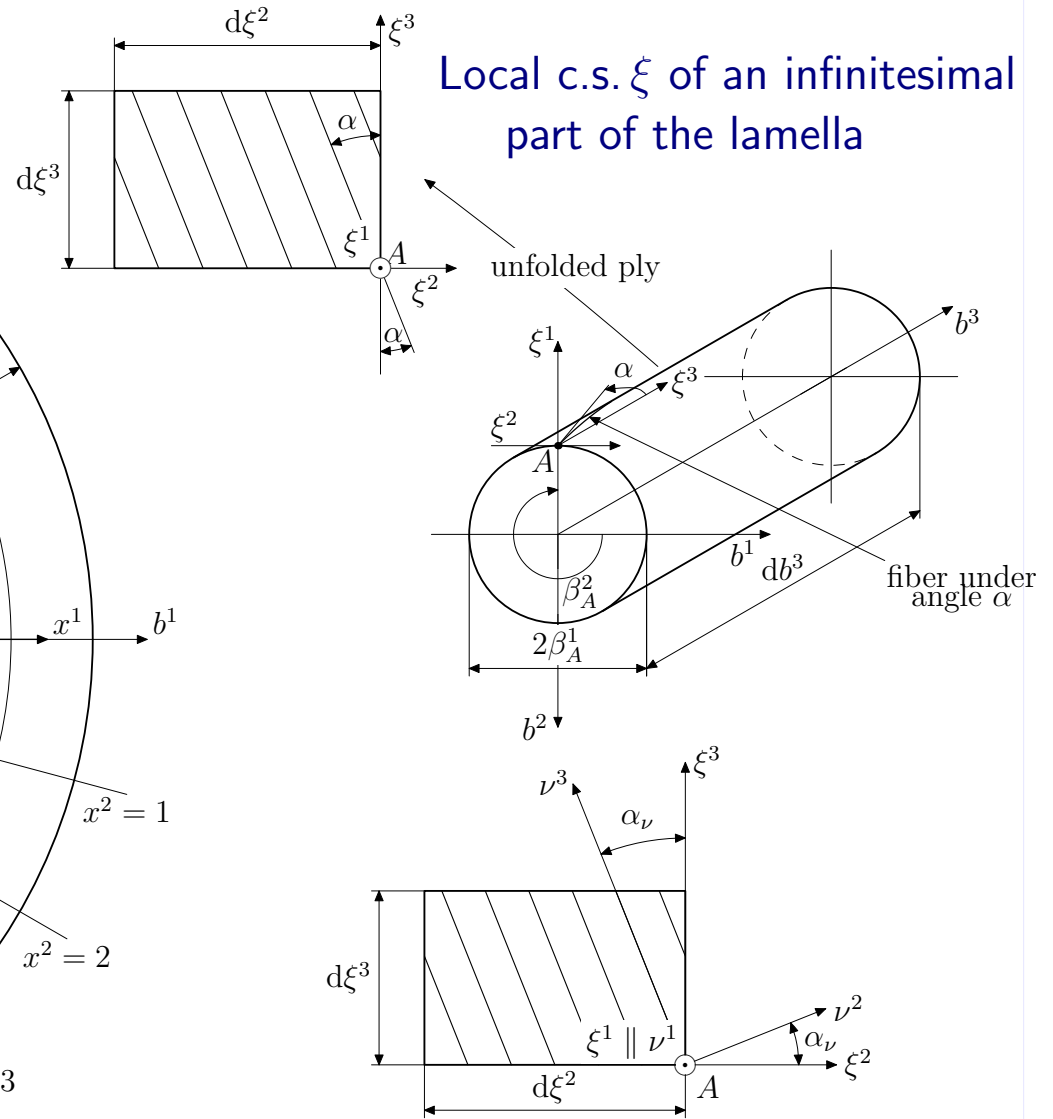
The computational frames of one osteon lamella

Osteon lamella

- ξ^a — Local Cartesian coordinate system
- b^a — Global Cartesian coordinate system of the osteon
- β^a — Global computational coordinate system of the osteon
- x^a — Coordinate system of the unfolded osteon lamella



Cortical bone osteon lamella coordinate system



Main material coordinate system ν^a of an unrolled infinitesimal part of the lamella

Metrics and Transformation rules

Via derivative of the relations between the coordinate systems we obtain a range of transformation matrices for the components of the tensors and range of metrics

$$\frac{\partial b^a}{\partial x^b} = \frac{\partial b^a}{\partial \beta^c} \frac{\partial \beta^c}{\partial x^b} = \begin{pmatrix} \cos \beta^2 & -\frac{\beta^1}{r_o} \sin \beta^2 & 0 \\ \sin \beta^2 & \frac{\beta^1}{r_o} \cos \beta^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

(LOVELOCK, 1989)

(SYNGE, 1978)

$$\frac{\partial b^a}{\partial \beta^b} = \begin{pmatrix} \cos \beta^2 & -\beta^1 \sin \beta^2 & 0 \\ \sin \beta^2 & \beta^1 \cos \beta^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}, \quad \frac{\partial x^a}{\partial \beta^b} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & r_o & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

metrics $ds^2 = g_{ab}^x dx^a dx^b = g_{ab}^\beta d\beta^a d\beta^b = g_{ab}^b db^a db^b = \delta_{ab} db^a db^b$

$$\delta_{ab} = g_{ab}^\xi = \frac{\partial x^c}{\partial \xi^a} \frac{\partial x^d}{\partial \xi^b} g_{cd}^x$$

$$\Rightarrow \frac{\partial x^a}{\partial \xi^b} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \frac{r_o}{\beta^1} & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

$$\frac{\partial \nu^a}{\partial \xi^b} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \cos \alpha_\nu & \sin \alpha_\nu \\ 0 & -\sin \alpha_\nu & \cos \alpha_\nu \end{pmatrix}$$

Inversely, from a known metric we can obtain the transformation rule

$$g_{ab}^x = \frac{\partial b^c}{\partial x^a} \frac{\partial b^d}{\partial x^b} \delta_{cd} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \left(\frac{\beta^1}{r_o}\right)^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

$$g_{ab}^\beta = \frac{\partial b^c}{\partial \beta^a} \frac{\partial b^d}{\partial \beta^b} \delta_{cd} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & (\beta^1)^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}$$

For the principal directions of an locally orthotropic block

Analysis of a cortical bone means analysis

Assemblage of the osteons

of the assemblage of the osteons embedded in isotropic interstitial matrix

to subtract
to add

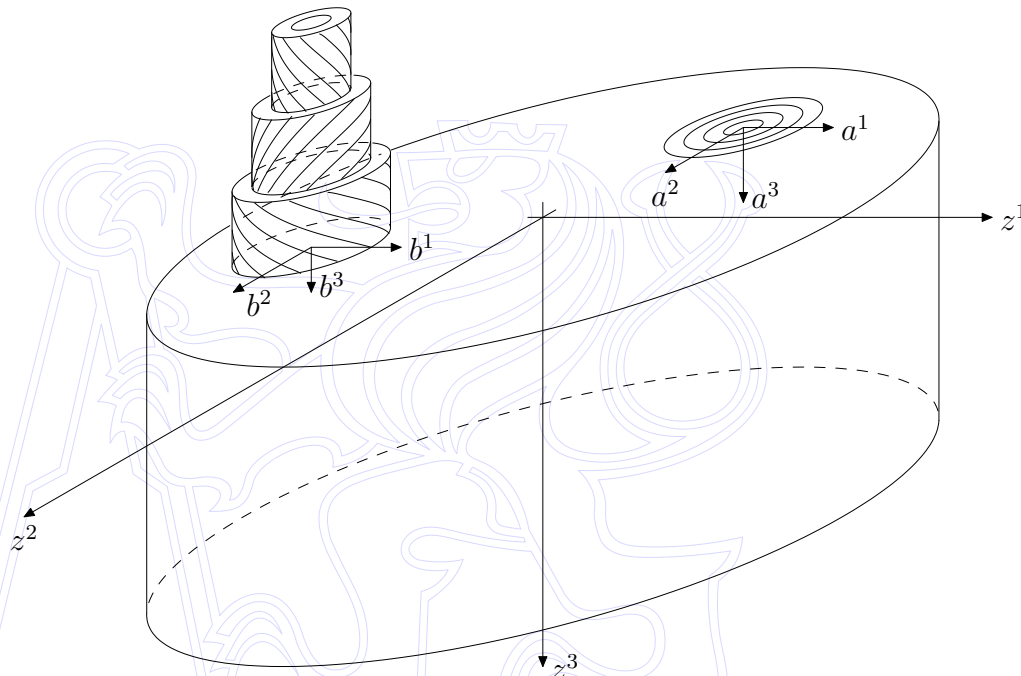
$$\hat{\mathbf{u}} = \arg \min_{\mathbf{u} \in \mathcal{Z}} \Pi(\mathbf{u}), \text{ kde } \Pi(\mathbf{u}) = a(\mathbf{u}, \mathbf{u}) - l(\mathbf{u})$$

$$a = \int_{\Omega} \varepsilon_{ab} \varepsilon^{cd} \mathcal{E}^{ab}_{cd} d\Omega + \sum_{\ell=1}^n \int_{\Omega_{\ell}} \varepsilon_{ab} \varepsilon^{cd} E^{ab}_{cd} d\Omega - \sum_{\ell=1}^n \int_{\Omega_{\ell}} \varepsilon_{ab} \varepsilon^{cd} \mathcal{E}^{ab}_{cd} d\Omega$$

(WASHIZU, 1975)

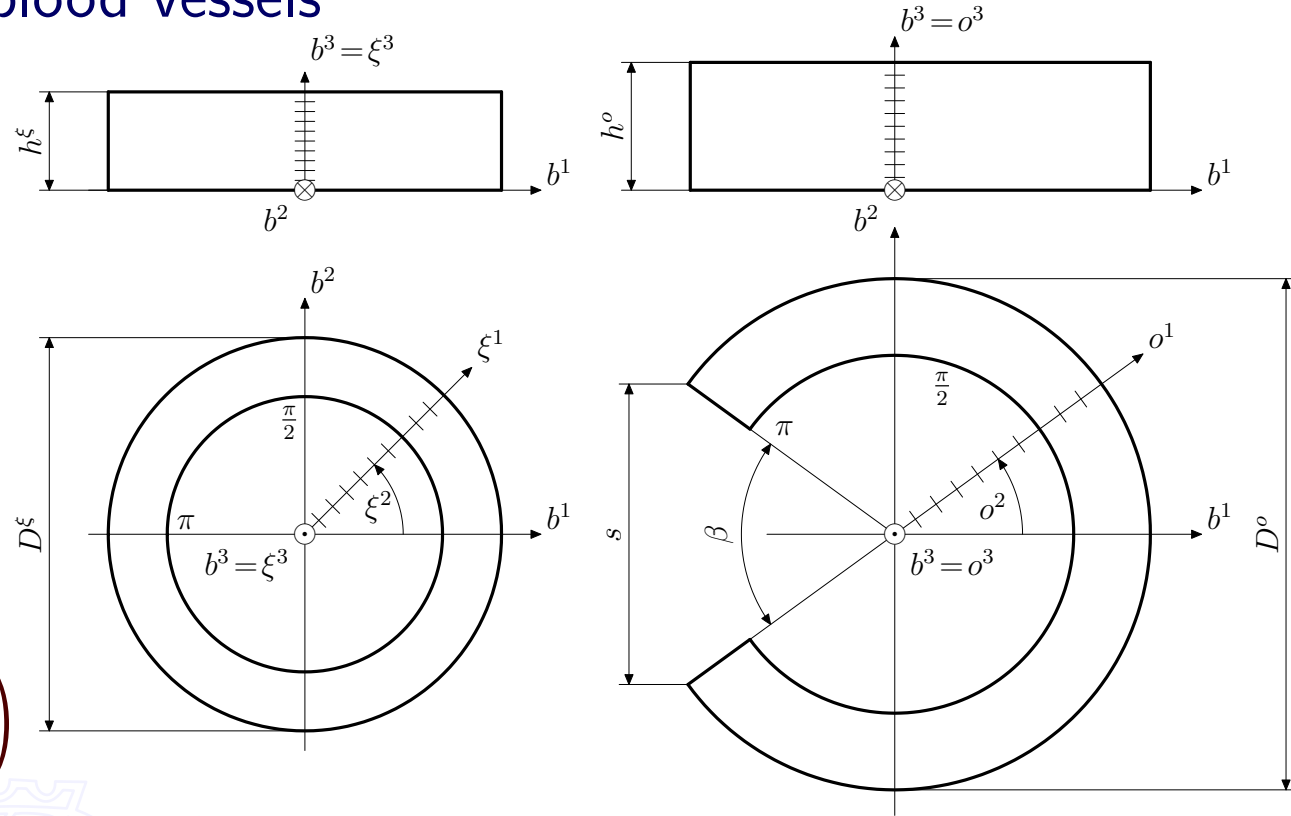
$$l(\mathbf{u}) = \int_{\Omega} p^i u_i d\Omega + \int_{\partial_t \Omega} t^i u_i d\Gamma$$

$$d\Omega = \left| g_{ab}^{\beta} \right|^{\frac{1}{2}} d^3 \beta, \quad d\Gamma = \left| h_{\alpha\beta}^{\phi} \right|^{\frac{1}{2}} d^2 \phi$$



Then the procedure is a similar one as in the previous examples

Residual stresses in blood vessels



$$\xi^1 = \delta o^1$$

$$\xi^2 = \gamma o^2$$

$$\xi^3 = \eta o^3$$

$$\delta = \frac{D^o}{D^\xi}, \quad \gamma = \frac{\pi - \frac{\beta}{2}}{\pi}, \quad \eta = \frac{h^o}{h^\xi}$$

$$\overset{o}{E}_{ab} = \frac{1}{2} \left(\overset{\xi}{g}_{ab} - \overset{o}{g}_{ab} \right)$$

$$\overset{o}{E}_{ab} = \frac{1}{2} \left(\overset{\xi}{g}_{ab} - \overset{o}{g}_{ab} \right) = \frac{1}{2} \begin{pmatrix} 1 - \delta^2 & 0 & 0 \\ 0 & (1 - \gamma^2) (\xi^1)^2 & 0 \\ 0 & 0 & 1 - \eta^2 \end{pmatrix}$$

$$\overset{\xi}{E}_{ab} = \frac{\partial o^c}{\partial \xi^a} \frac{\partial o^d}{\partial \xi^b} \overset{o}{E}_{cd} = \frac{1}{2} \begin{pmatrix} \frac{1 - \delta^2}{\delta^2} & 0 & 0 \\ 0 & \frac{1 - \gamma^2}{\gamma^2} (\xi^1)^2 & 0 \\ 0 & 0 & \frac{1 - \eta^2}{\eta^2} \end{pmatrix}$$

$$\overset{\xi}{S}^{ab} = E^{abcd} \overset{\xi}{E}_{cd} = \left(\lambda \overset{\xi}{g}^{ab} \overset{\xi}{g}^{cd} + \mu \overset{\xi}{g}^{ac} \overset{\xi}{g}^{bd} + \mu \overset{\xi}{g}^{ad} \overset{\xi}{g}^{bc} \right) \overset{\xi}{E}_{cd}$$

$$\overset{t}{S}^{ab} = \frac{\partial t^a}{\partial \xi^c} \frac{\partial t^b}{\partial \xi^d} \overset{\xi}{S}^{cd}$$

Stiffness maximization

BENDSØE, M. P. (2003)

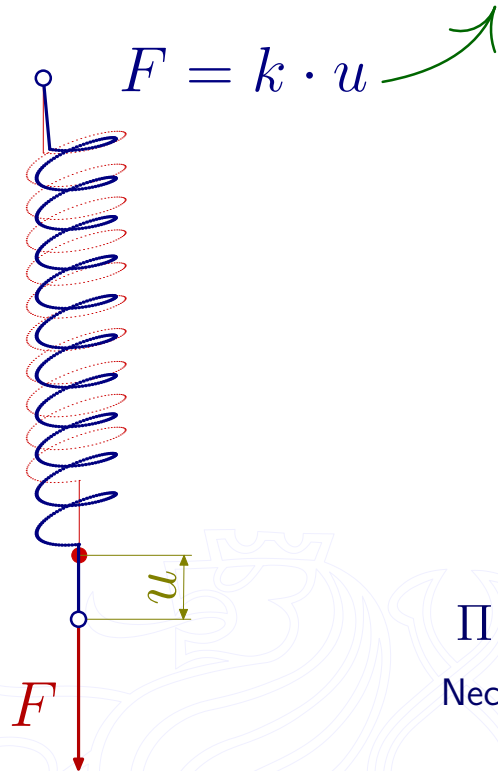
MAREŠ, T. (2006)

ALLAIRE, G. (2002)

MAREŠ, T. (2005)

Minimum work of external forces \Leftrightarrow Maximum stiffness

$$\min F \cdot u \Leftrightarrow \min \frac{F^2}{k} \Leftrightarrow \max k$$



$$\min \Pi = \frac{1}{2}(A\hat{u}, \hat{u}) - (f, \hat{u}) \wedge (A\hat{u}, \hat{u}) = (f, \hat{u})$$

$$\Rightarrow \min \Pi = -\frac{1}{2}(f, \hat{u})$$

$$\Rightarrow \arg \min (f, \hat{u}) = \max \min \Pi$$

$$\{\hat{\mathbf{E}}, \hat{\mathbf{u}}\} = \arg \max_{\mathbf{E} \in} \min_{\mathbf{u} \in} \Pi(\mathbf{u}, \mathbf{E})$$

$$\Pi = \Pi(w, \alpha)$$

Necessary condition $\frac{\delta \Pi}{\delta w} = 0, \frac{\partial \Pi}{\partial \alpha} = 0$

Alternative fulfilment of the necessary condition

etc. until convergence

0. Choose α^0

1. The elasticity problem $\frac{\partial \Pi(w, \alpha^k)}{\partial w} = 0 \Rightarrow w^{k+1} \rightarrow$ 2. The opt. condition $\frac{\partial \Pi(w^{k+1})}{\partial \alpha} = 0 \Rightarrow \alpha^{k+1}$

w — Fourier series coefficients

Saddle point implies convergence

Stress variant of the stiffness maximization problem

BENDSØE, M. P. (2003)

MAREŠ, T. (2006)

ALLAIRE, G. (2002)

$$\{\hat{\mathbf{E}}, \hat{\mathbf{u}}\} = \arg \max_{\mathbf{E} \in \mathcal{E}} \min_{\mathbf{u} \in \mathcal{U}} \Pi(\mathbf{u}, \mathbf{E})$$

$$\Pi = \frac{1}{2} \int_{\Omega} E_{ijkl} \varepsilon_{ij} \varepsilon_{kl} \, d\Omega - \int_{\Omega} p_i u_i \, d\Omega - \int_{\partial_t \Omega} t_i u_i \, dS$$

$$\sigma_{ij} = E_{ijkl} \varepsilon_{kl}$$

$$\varepsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i})$$

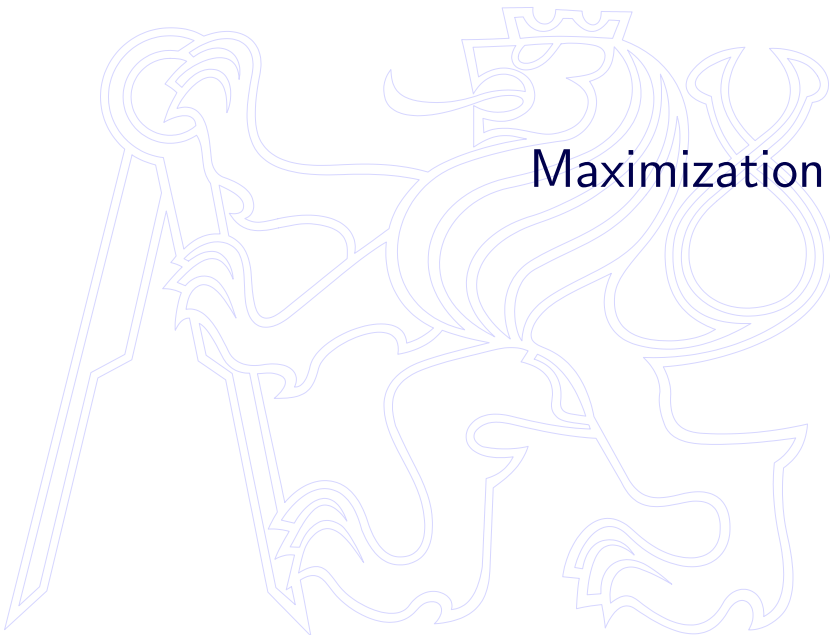
$$\{\hat{\mathbf{C}}, \hat{\boldsymbol{\sigma}}\} = \arg \min_{\mathbf{C} \in \mathcal{C}} \min_{\boldsymbol{\sigma} \in \mathcal{S}} \frac{1}{2} \int_{\Omega} C_{ijkl} \sigma_{ij} \sigma_{kl} \, d\Omega$$

$$= \{\sigma_{ij} \mid \sigma_{ij,i} + p_j = 0 \text{ na } \Omega \wedge \sigma_{ij} \ell_j = t_i \text{ na } \partial_t \Omega\}$$

Maximization under uncertain conditions

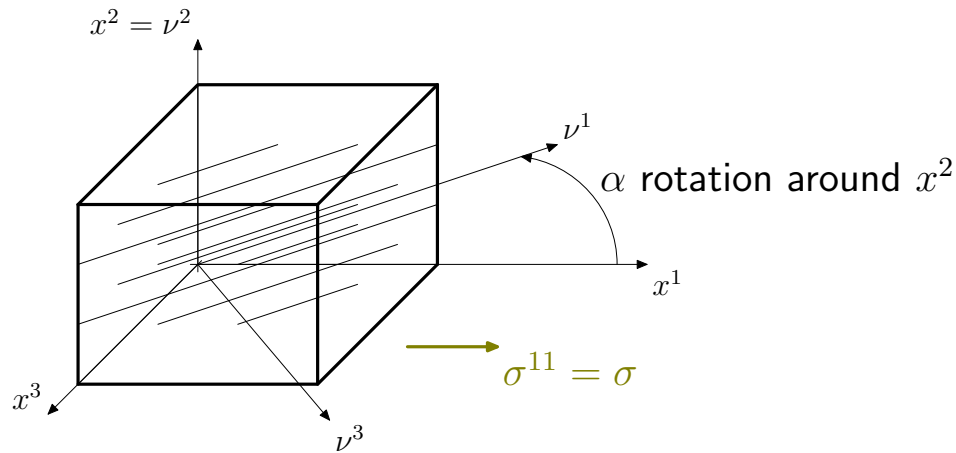
$$\{\hat{\mathbf{C}}, \hat{\mathbf{t}}, \hat{\boldsymbol{\sigma}}\} = \arg \min_{\mathbf{C} \in \mathcal{C}} \max_{\mathbf{t} \in \mathcal{T}} \min_{\boldsymbol{\sigma} \in \mathcal{S}} \int_{\Omega} C_{ijkl} \sigma_{ij} \sigma_{kl} \, d\Omega$$

— the set of possible loading states



The simplest (illustrating) problem of fibre composite stiffness maximization

MAREŠ, T. (2009)



$$\sigma_{ab}^x = \begin{pmatrix} \sigma & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\varepsilon_{ab}^\nu = C_{abcd}^\nu \sigma^{cd}$$

$$\left\{ C_{abcd}^\nu \right\}_{\{ab\}[cd]} = \begin{pmatrix} \frac{1}{E_{11}} & 0 & 0 & 0 & -\frac{\nu_{21}}{E_{22}} & 0 & 0 & 0 & -\frac{\nu_{31}}{E_{33}} \\ 0 & \frac{1}{4G_{12}} & 0 & \frac{1}{4G_{12}} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 \\ 0 & \frac{1}{4G_{12}} & 0 & \frac{1}{4G_{12}} & 0 & 0 & 0 & 0 & 0 \\ -\frac{\nu_{12}}{E_{11}} & 0 & 0 & 0 & \frac{1}{E_{22}} & 0 & 0 & 0 & -\frac{\nu_{32}}{E_{33}} \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{4G_{23}} & 0 & \frac{1}{4G_{23}} & 0 \\ 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 & 0 & \frac{1}{4G_{13}} & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{1}{4G_{23}} & 0 & \frac{1}{4G_{23}} & 0 \\ -\frac{\nu_{13}}{E_{11}} & 0 & 0 & 0 & -\frac{\nu_{23}}{E_{22}} & 0 & 0 & 0 & \frac{1}{E_{33}} \end{pmatrix}$$

$$\min_{\alpha} \int_V c dV$$

$$c = \sigma^{ab} C_{abcd}^x \sigma^{cd} = \sigma C_{1111}^x \sigma$$

The transformation of the compliance tensor and results

$$C_{abcd}^x = \frac{\partial \nu^i}{\partial x^a} \frac{\partial \nu^j}{\partial x^b} \frac{\partial \nu^k}{\partial x^c} \frac{\partial \nu^l}{\partial x^d} C_{ijkl}^\nu$$

$$\frac{\partial \nu^i}{\partial x^a} = \begin{pmatrix} \cos \alpha & 0 & -\sin \alpha \\ 0 & 1 & 0 \\ \sin \alpha & 0 & \cos \alpha \end{pmatrix}$$

$$c = \sigma^{ab} C_{abcd}^x \sigma^{cd} = \sigma C_{1111}^x \sigma$$

$$C_{1111}^x = \frac{\partial \nu^i}{\partial x^1} \frac{\partial \nu^j}{\partial x^1} \frac{\partial \nu^k}{\partial x^1} \frac{\partial \nu^l}{\partial x^1} C_{ijkl}^\nu$$

$$C_{1111}^x = \left(\cos^2 \alpha \quad 0 \quad \cos \alpha \sin \alpha \quad 0 \quad 0 \quad 0 \quad \sin \alpha \cos \alpha \quad 0 \quad \sin^2 \alpha \right) \left\{ C_{abcd}^\nu \right\}_{\{ab[cd\}}$$

$$\begin{pmatrix} \cos^2 \alpha \\ 0 \\ \cos \alpha \sin \alpha \\ 0 \\ 0 \\ 0 \\ \sin \alpha \cos \alpha \\ 0 \\ \sin^2 \alpha \end{pmatrix}$$

$$c = \sigma^2 \left(\cos^4 \alpha \frac{1}{E_{11}} + \cos^2 \alpha \sin^2 \alpha \left(\frac{1}{G_{13}} - \frac{\nu_{31}}{E_{11}} - \frac{\nu_{13}}{E_{33}} \right) + \sin^4 \alpha \frac{1}{E_{33}} \right)$$

$$\frac{\partial c}{\partial \alpha} = 0$$

$$\cos^3 \alpha \sin \alpha A_1 + \cos \alpha \sin^3 \alpha A_2 = 0$$

$$A_1 = \frac{1}{G_{13}} - \frac{\nu_{31}}{E_{11}} - \frac{\nu_{13}}{E_{33}} - \frac{2}{E_{11}}$$

$$A_2 = \frac{2}{E_{22}} + \frac{\nu_{31}}{E_{11}} + \frac{\nu_{13}}{E_{33}} - \frac{1}{G_{13}}$$

$$\hat{\alpha}_1 = \pm \frac{\pi}{2} \text{ with } c_1 = \frac{\sigma^2}{E_{33}}$$

$$\hat{\alpha}_2 = 0, \pi \text{ with } c_2 = \frac{\sigma^2}{E_{11}}$$

$$\hat{\alpha}_{3,4} = \arctan \left(\pm \sqrt{-\frac{A_1}{A_2}} \right)$$

The elasticity problem

$$P^{abcd} w_{cd} = q^{ab}$$

The necessary condition of optimum

$$w_{ab} w_{cd} R^{abcd}(\alpha_\nu) = 0$$

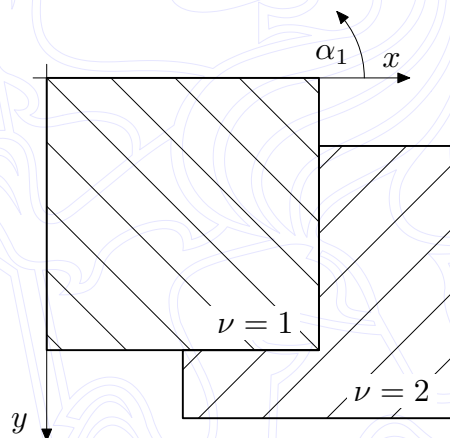
Laminated multilayer Kirchhoff plates of symmetric layout

$R^{abcd}(\alpha_\nu)$ functions of the design parameters

α_ν stands for the layer orientation

w_{ab} Fourier series expansion coefficients of the perpendicular displacement

q^{ab} Fourier series expansion coefficients of the load



$$\alpha_1 = -45^\circ$$

$$\alpha_2 = 45^\circ$$

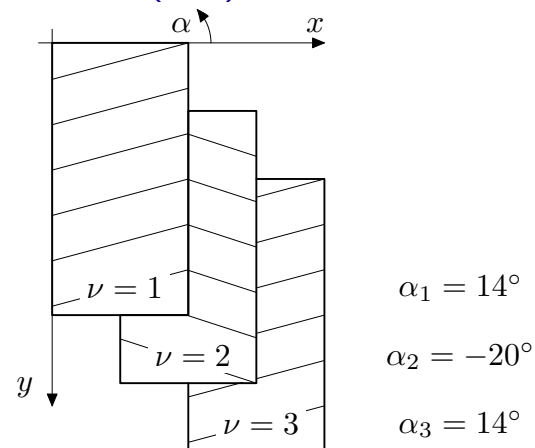
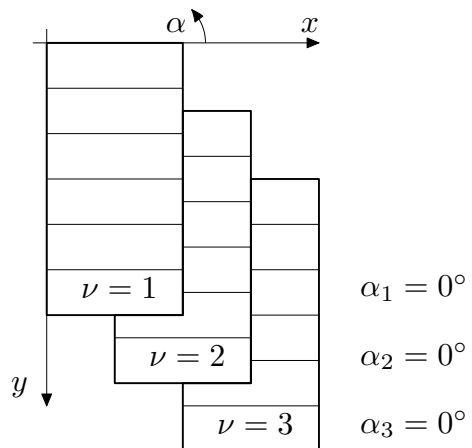
Any permutation of the layout is possible

The alternative fulfilment of the necessary condition

Square plate of four layers loaded by $q = q_0 \sin \frac{\pi x}{a} \sin \frac{\pi y}{b}$

The layout maximizing the stiffness

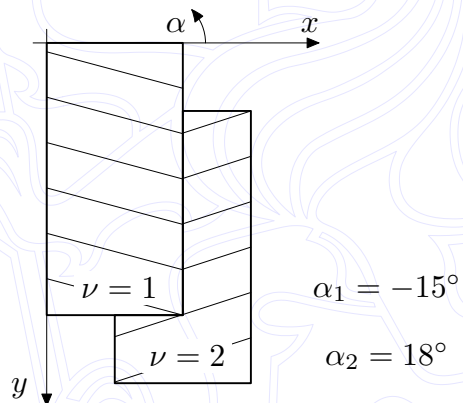
Rectangular plate (1:2) of six layers loaded by $q = q_0xy$



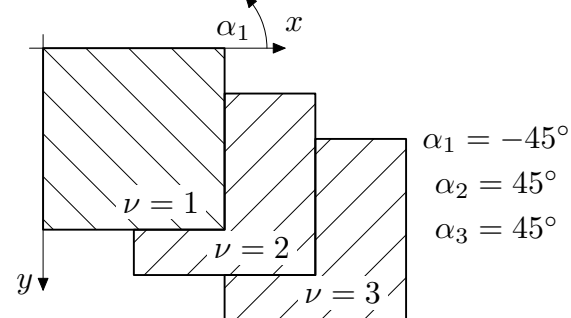
Rectangular plate (1:2) of six layers loaded by $q = q_0 \sin \frac{\pi x}{a} \sin \frac{\pi y}{b}$

Any permutation of the layout is possible

Rectangular plate (1:2) of four layers loaded by $q = q_0xy$



Square plate of six layers loaded by $q = q_0 \sin \frac{2\pi x}{a} \sin \frac{2\pi y}{b}$

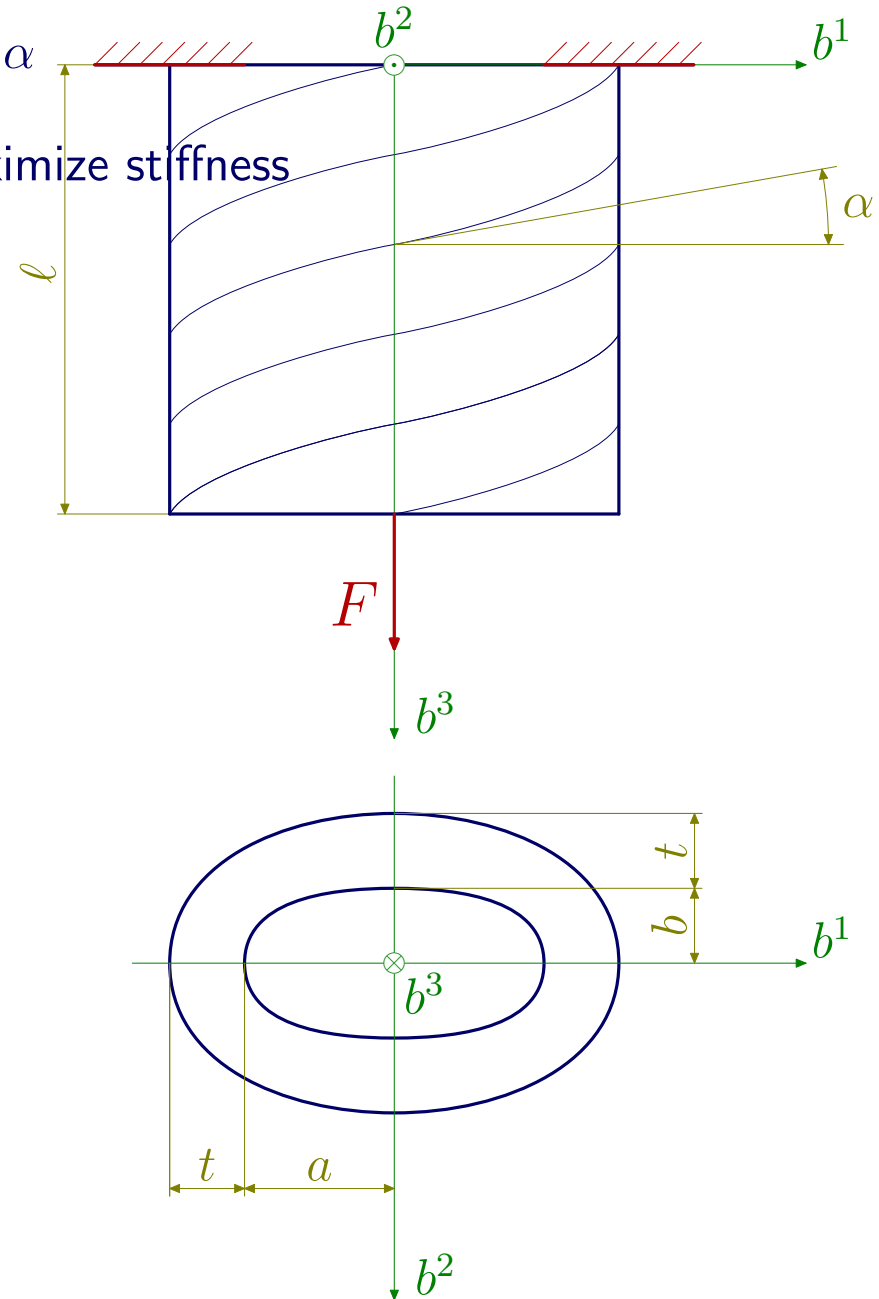
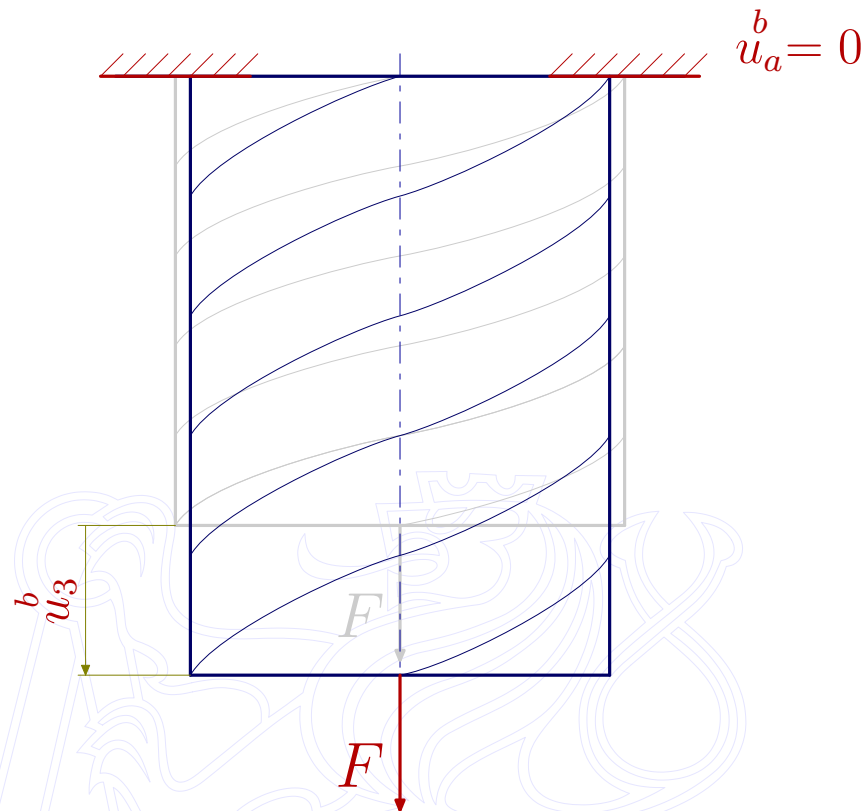


The alternative fulfilment of the necessary condition

The problem

The thick-walled elliptic tube coiled with an angle α loaded with Force F and clamped as seen at Fig.

The question is how to choose the angle α to maximize stiffness



The equation, right hand side and solution

$$\frac{\partial a}{\partial A} = \frac{\partial l}{\partial A} \quad \frac{\partial a}{\partial A} = KA \quad KA = P$$

$$l = \int_S \frac{F}{S} u_3 \, dS$$

$$l = \int_0^{2\pi} \int_0^t \frac{F}{S} [\text{zeros}(1,343), \text{zeros}(1,343), \text{phi}] * \\ * \text{sqrt}(\det(\text{gx})) \, dx^1 dx^2 * A$$

$$P = \frac{\partial l}{\partial A} = \begin{pmatrix} \text{zeros}(363) \\ \text{zeros}(363) \\ \int_0^{2\pi} \int_0^t \frac{F}{S} \text{phi}' * \text{sqrt}(\det(\text{gx})) \, dx^1 dx^2 \end{pmatrix}$$

x1=...; x2=...; x3=...

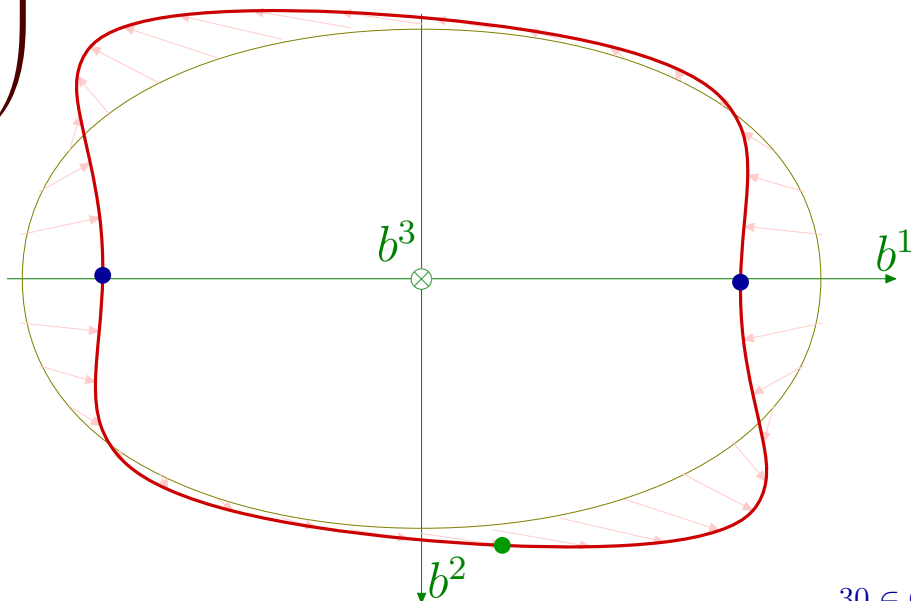
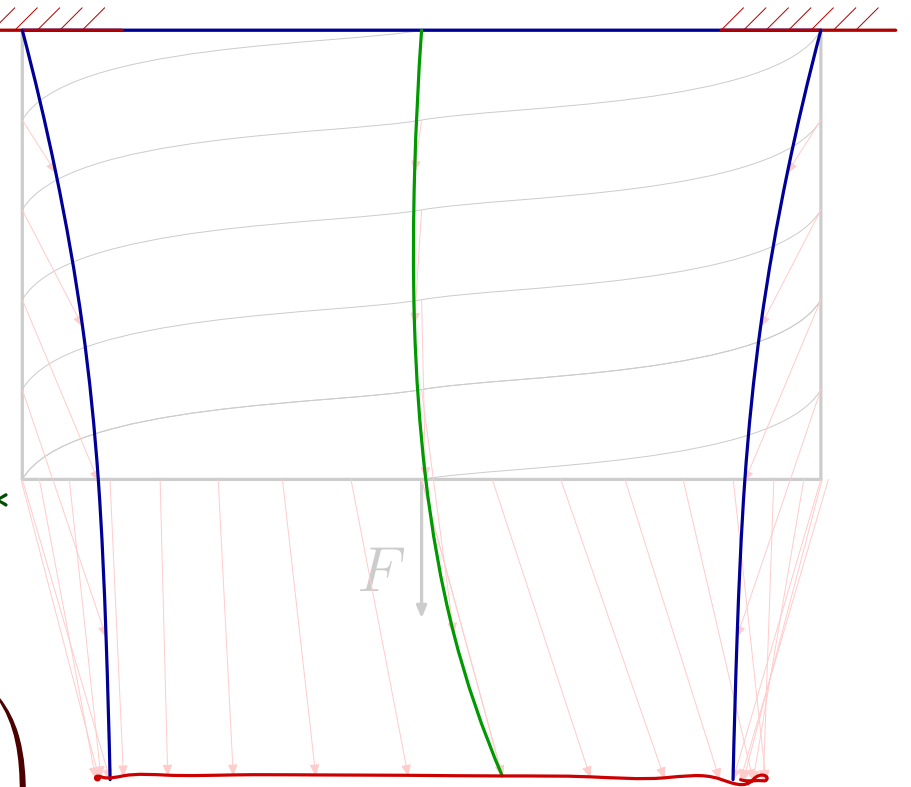
A=K*(-1)*P

phi=x3*kron(kron(exp(i*j*x1*2*pi/t), ...

ux=real([phi,zeros(1,siz),zeros(1,...

xb=1/(a*(sin(x2))**2+b*(cos(x2))**2+...

ub=xb*ux



Alternative fulfilment of the necessary condition of SM

BENDSØE, M. P. (2003)
 MAREŠ, T. (2006)
 ALLAIRE, G. (2002)

0. Choose an angle α

1. The problem of elasticity as already solved

$$\mathbf{A} = \mathbf{K}^{-1}\mathbf{P}$$

2. The stiffness maximum condition,

$$\frac{\partial \Pi}{\partial \alpha} = 0, \text{ i.e., } \frac{1}{2} \mathbf{A}^T \frac{\partial \mathbf{K}}{\partial \alpha} \mathbf{A} = 0$$

$$\frac{\partial \mathbf{K}}{\partial \alpha} = \int_0^\ell \int_0^{2\pi} \int_0^t (\mathbf{B}\text{-Gam})^* \frac{\partial \mathbf{E}\mathbf{x}}{\partial \alpha} (\mathbf{B}\text{-Gam}) \sqrt{\det(\mathbf{g}\mathbf{x})} d^3x$$

$$\frac{\partial \mathbf{E}\mathbf{x}}{\partial \alpha} = \left\{ \frac{\partial E^{abcd}}{\partial \alpha} \right\}_{ab[cd]}$$

$$\frac{\partial E^{abcd}}{\partial \alpha} = \left(\alpha_i^a \frac{\partial x^b}{\partial \nu^j} \frac{\partial x^c}{\partial \nu^k} \frac{\partial x^d}{\partial \nu^l} + \frac{\partial x^a}{\partial \nu^i} \alpha_j^b \frac{\partial x^c}{\partial \nu^k} \frac{\partial x^d}{\partial \nu^l} + \frac{\partial x^a}{\partial \nu^i} \frac{\partial x^b}{\partial \nu^j} \alpha_k^c \frac{\partial x^d}{\partial \nu^l} + \frac{\partial x^a}{\partial \nu^i} \frac{\partial x^b}{\partial \nu^j} \frac{\partial x^c}{\partial \nu^k} \alpha_l^d \right) E^{\nu jkl}$$

$$\alpha_b^a = \frac{\partial}{\partial \alpha} \left[\frac{\partial x^a}{\partial \nu^b} \right] = \frac{\partial x^a}{\partial b^c} \frac{\partial b^c}{\partial \xi^d} \frac{\partial}{\partial \alpha} \left[\frac{\partial \xi^d}{\partial \nu^b} \right]$$

3. go to item 1

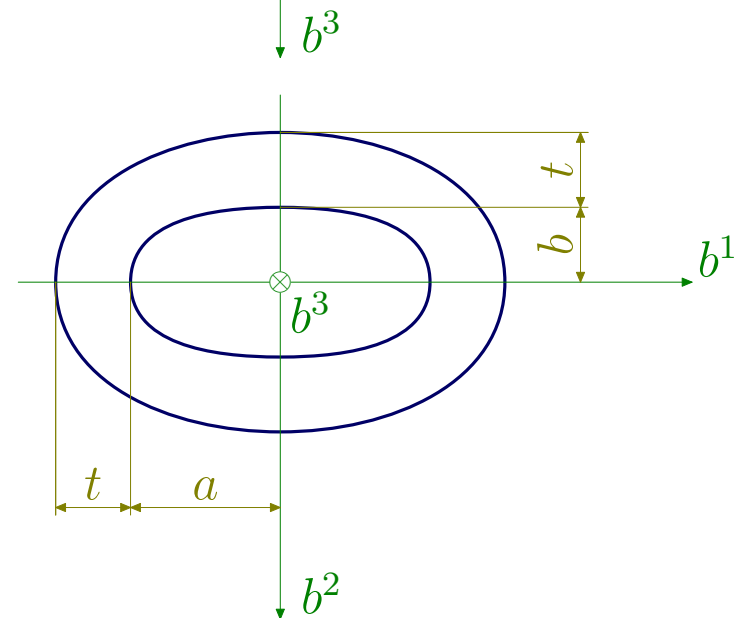
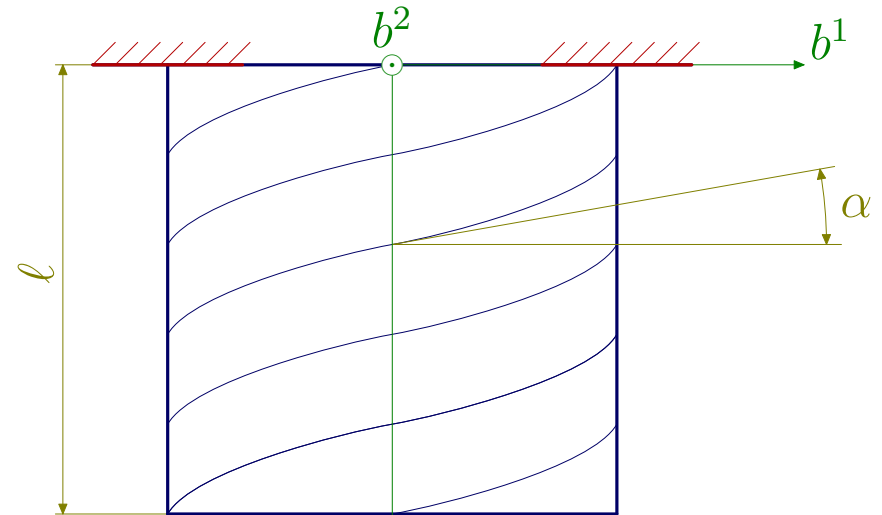
The results of the problem

The optimum angle α for given loadings

For F_3 $\alpha_{\text{opt}} = 90^\circ$

For T_1 $\alpha_{\text{opt}} = 0^\circ$

For T_2 $\alpha_{\text{opt}} = 45^\circ$



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